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Massachusetts Institute of Technology

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ON THE NATURAL TRANSVERSE MODES  
OF A SUBMARINE

---

Edward John Otth  
and  
Peter Wright Wood

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U. S. Naval Postgraduate School  
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ON THE NATURAL TRANSVERSE MODES  
OF A SUBMARINE

by

EDWARD JOHN OTTH, JR. Lieutenant, U. S. Navy  
B. S., U. S. Naval Academy (1949)

and

PETER WRIGHT WOOD, Lieutenant (junior grade), U. S. Navy  
B. S., U. S. Naval Academy (1950)

SUBMITTED IN PARTIAL FULFILLMENT OF THE  
REQUIREMENTS FOR THE DEGREE OF  
NAVAL ENGINEER

at the

MASSACHUSETTS INSTITUTE OF TECHNOLOGY

May, 1955



ABSTRACT

## ON THE NATURAL TRANSVERSE MODES OF A SUBMARINE

by

EDWARD JOHN OTTH

and

PETER WRIGHT WOOD

SUBMITTED TO THE DEPARTMENT OF NAVAL ARCHITECTURE  
AND MARINE ENGINEERING ON MAY 23, 1955, IN PARTIAL  
FULFILLMENT OF THE REQUIREMENTS FOR THE DEGREE OF  
NAVAL ENGINEER.

A graphical method of determining normalized curvature patterns, deflection patterns, and frequencies for the natural modes of interest of a ship treated as a non-uniform, free-free, transversely vibrating beam is presented. Corrections for shear deflection and rotatory inertia are integrated into the method, which is based on procedures developed by Stodola and Schadlofsky. Integrations are performed mechanically by an integrator. The method is demonstrated by determining the first three horizontal modes of a three-eighths scale model of a submerged submarine.

The method is found to be convergent, yielding acceptable results, and taking account of the secondary effects considered. Three cycles of iteration for each of the first three non-trivial natural modes of the model yielded frequencies of 417, 838, and 1391 cycles per minute, respectively. Patterns obtained are tabulated in detail.

The process developed is recommended for consideration by small research facilities with limited budgets.

Thesis Supervisor: STEPHEN HARRY CRANDALL  
Ph. D.

Title: Associate Professor of Mechanical Engineering



[illegible]

WTTG 5801 08A069

NAVY ENGINEER  
TWO-THIRDS OF THE REQUIREMENTS FOR THE DESIGN OF  
AND MAKE MODIFICATIONS ON MAY 11, 1958, IN PARTIAL  
SUBMITTED TO THE DEPARTMENT OF NAVAL ARCHITECTURE

A graphical method of determining normalized curvature, deflection patterns, and frequencies for the natural modes of interest of a ship is presented. Corrections for shear transversely bending beam is presented. Corrections for shear deflection and rotary inertia are integrated into the method, which is based on procedures developed by Stodola and Bessodinsky. Integrations are performed mechanically by an integrator. The method is demonstrated by determining the first three horizontal modes of a three-sigma scale model of a submerged submarine.

The method is found to be consistent, yielding reproducible results, and taking account of the secondary effects considered. Three cycles of iteration for each of the first three and (initial) natural modes of the model yielded frequencies of 41.7, 52.8, and 137.1 cycles per minute, respectively. Values obtained are tabulated in detail.

The process developed is recommended for consideration by small research facilities with limited budgets.

The 1997/98 Season of Mechanical Engineering

Cambridge, Mass.  
May 23, 1955

Secretary of the Faculty  
Massachusetts Institute of Technology  
Cambridge 39, Massachusetts

Dear Sir:

In accordance with the requirements for the Degree of Naval Engineer, we herewith submit a thesis entitled: "On the Natural Transverse Modes of a Submarine".

Respectfully yours,



### ACKNOWLEDGMENT

This thesis was undertaken at the suggestion of Dr. A. H. Keil, Dr. H. M. Schauer, and Mr. W. W. Murray of Underwater Explosions Research Division, Norfolk Naval Shipyard, Norfolk, Virginia. The authors are grateful to Mr. Murray, who outlined the basic approach, formulated several important relationships, and guided preliminary investigations which preceded this thesis; to Lieutenant Commander R. E. Henning, U.S. Navy, officer-in-charge, Underwater Explosions Research Division, for his cooperation in making available structural data and previously calculated frequencies for the model considered; to Professor J. P. Den Hartog for his encouragement and guidance during the early stages of the thesis work; to Professor S. H. Crandall, for his patient guidance and assistance throughout the execution of this thesis; and to Miss Jill Littlefield and Miss Edith Repshis for carefully and neatly typing the manuscript and tables.





# NOMENCLATURE

- A = total cross-section area of hull structure considered effective in bending and shear, (sq. inches)
- $\bar{A}$  = axis translation of assumed or calculated deflection for orthogonality with rigid body translation
- $\bar{B}$  = rotation coefficient for axis of assumed or calculated deflection for orthogonality with rigid body rotation
- $C_1, C_{11} \dots$  = adjustment coefficients for correcting assumed or calculated deflections for orthogonality with previous modes
- $C_1$  = constant of integration applied to shear (equal to zero)
- $C_2$  = constant of integration applied to bending (equal to zero)
- $C_3$  = constant of integration applied to slope
- $C_4$  = constant of integration applied to deflection
- E = Young's modulus,  $30 \times 10^6$  psi
- G = shear modulus,  $11.5 \times 10^6$  psi
- $I, I_{zz}$  = moment of inertia of hull structure about vertical axis through centroid of each section.
- K = ratio of average shear stress to maximum shear stress over a cross-section for horizontal transverse shear.
- L = length overall of ship, ft.
- M = bending moment, ft-lb.
- N = an operational constant, 
$$= \int_0^L \frac{(x - \bar{x})^2}{L} m g \frac{dx}{L}$$

FORMULAE

- A = total cross-section area of hull structure considered effective in bending and shear, (sq. inches)
- $\bar{I}$  = area moment of inertia of uncutted section for bending flexure with rigid web connection
- $\bar{I}_0$  = section moment of inertia of uncutted section flexure with rigid web connection
- $\bar{I}_{00}$  = adjusted coefficient for flexure section of uncutted section for rigidity with rigid web connection
- $C_1$  = constant of integration applied to shear (page 10)
- $C_2$  = constant of integration applied to bending (page 10)
- $C_3$  = constant of integration applied to flexure
- B = Timmer's modulus,  $B_0 = 10^6 \text{ psi}$
- D = shear modulus,  $D_0 = 10^6 \text{ psi}$
- $I_{22}$  = moment of inertia of hull structure about vertical axis through center of mass
- E = ratio of average shear stress to maximum shear stress over a cross-section for bottomway pressure load
- F = length overall of ship, ft.
- M = bending moment, ft.-lb.
- N = an operational constant,  $N = \frac{1}{2} \left( \frac{10 - \bar{I}}{10} \right) \frac{B_0}{D_0}$

NOMENCLATURE (Continued)

- $P$  = an operational constant,  $= \int_0^L \int_0^x \frac{(\xi - \bar{x})}{L} m g d\xi \frac{1}{L} \frac{dx}{L}$
- $Q$  = shear force, lb
- $T$  = period of vibration, milliseconds
- $W_1$  = an operational constant  $= \int_0^L \bar{y}_1^2 m g dx/L$
- $W_2$  = an operational constant  $= \int_0^L \bar{y}_2^2 m g dx/L$
- $Y_1, Y_2$  = deflection patterns for a uniform prismatic bar obtained from consideration of simple bending theory only, according to Lord Rayleigh.
- $f$  = frequency of vibration, cps
- $g$  = force of gravity, 32.2 ft/sec<sup>2</sup>
- $m$  = virtual mass per unit length, including structure, ballast, sea-flooding water, etc., plus water entrained in horizontal vibration (lb sec<sup>2</sup>/ft<sup>2</sup>)
- $w$  =  $m g$ , virtual weight per unit length (lb/ft)
- $x$  = coordinate parallel to longitudinal axis of ship, measured from bow (ft)
- $\bar{x}$  = coordinate of longitudinal centroid of virtual mass
- $y$  = coordinate parallel to beam of ship, measured from vertical plane through centroid of ship; measure of horizontal deflection
- $\bar{y}$  = the deflection pattern obtained from a cycle, before orthogonalization
- $y^*$  = value of unnormalized deflection curve taken at the normalizing point



MECHANICAL ANALYSIS (continued)

1.  $\frac{d^2x}{dt^2} = -\frac{g}{L}x$  is a differential equation of the second order, linear, homogeneous, with constant coefficients.

2.  $\frac{d^2x}{dt^2} = -\frac{g}{L}x$  is a differential equation of the second order, linear, homogeneous, with constant coefficients.

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14.  $\frac{d^2x}{dt^2} = -\frac{g}{L}x$  is a differential equation of the second order, linear, homogeneous, with constant coefficients.

15.  $\frac{d^2x}{dt^2} = -\frac{g}{L}x$  is a differential equation of the second order, linear, homogeneous, with constant coefficients.

# NOMENCLATURE Continued

- $\bar{z}$  = coördinate parallel to vertical axis of ship measured from plane through centroid of ship.
- $\Delta$  = virtual displacement (pounds); see
- $\beta$  = component of slope due to shear deflection
- $\gamma$  = component of slope due to bending
- $\bar{\gamma}$  = component of slope due to bending before axis determination;  $\bar{\gamma} + c_3 = \gamma$
- $\eta$  = deflection before determination of axis:  $\eta + c_4 = y$
- $\eta'$  = slope before determination of axis;  $\eta + c_3 = y'$
- $\xi$  = dummy variable, replacing  $x$
- $\omega$  = circular frequency, (radians/second)

Note: Subscript to deflection indicates mode number.

Hyphenated subscript indicates mode number and iterative cycle number.

Prime indicates differentiation with respect to  $x$ .

# WORKSHEET 1: THE CONTENTS

1. coefficient of correlation  $r$  is the ratio of the covariance to the product of the standard deviations of the two variables.

2.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

3.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

4.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

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17.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

18.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

19.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

20.  $r = \frac{\text{covariance}(x, y)}{\sigma_x \sigma_y}$

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## I. INTRODUCTION

### A. Historical

For the last seventy years, the subject of vibrations of ship hulls has attracted ever increasing attention from the naval architect. Early investigators, notably Otto Schlick, emphasized the empirical treatment; but so complex is the structure of a large ship, and so varied its design features, that it became clear that this approach would not fully serve the need. Ultimately then, we must look to the theoretical approach for the development of adequate formulations.

Basically, the ship is considered a slender free-free elastic beam, vibrating transversely to its long axis, for which the theory stems from Lord Rayleigh. [6]\* From energy considerations he developed the basic equation based on simple bending. He then showed that for various assumed mode shapes, the one for which the energy balance gave the lowest frequency was the correct shape. He also published a modified equation, accounting for the effect of rotatory inertia, but, considering this effect negligible, did not emphasize it.

Timoshenko formulated the shear flexibility term for Rayleigh's basic equation, and this set down the well known Timoshenko mechanism, which considers the contribution of bending, shear, and rotatory inertia. While this formulation is the most complete available, it is complex and difficult to solve exactly, even for a uniform beam, and in recent years a number of papers have been devoted to this solution, generally involving Laplace transforms or series solutions. 11, 33, 34, 63, et.al.

Interest in the non-uniform beam inspired work along different lines, since a beam with a general distribution of mass, inertia, and section area is not amenable to exact solution. One approach is to attempt to formulate one or more of these distributions into expressions suitable for analytic

\* Numbers in brackets refer to references listed in Appendix F.

# I. INTRODUCTION

## A. Historical

For the last twenty years, the subject of vibrations of ship hulls has attracted ever increasing attention from the naval architect. Early investigators, notably G. B. Smith, emphasized the physical treatment and as compared to the structure of a large ship, and so varied its design features, that it became clear that this approach would not fully serve the need. Unfortunately then, we must look to the theoretical approach for the development of adequate formulations.

Essentially, the ship is considered a slender isotropic elastic beam, vibrating transversely to its long axis, for which the theory stems from Lord Rayleigh. [1] From energy considerations he developed the basic equation based on simple bending. He then showed that for various assumed mode shapes, the one for which the energy balance gave the lowest frequency was the correct shape. He also obtained a modified equation accounting for the effect of rotary inertia, but, neglecting this effect, negligible, did not emphasize it.

Timoshenko formulated the basic differential equation for Rayleigh's basic equation, and this is done the well known Timoshenko modification which considers the composition of bending, shear, and rotary inertia. While this formulation is the most complete available, it is complex and difficult to solve exactly, even for a uniform beam, and in recent years a number of papers have been devoted to this problem, generally involving Laplace transforms or matrix solutions. [2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100]

Interest in the non-uniform beam problem was also growing then, since a beam with a general distribution of mass, inertia, and section area is not amenable to exact solution. One approach is to attempt to represent one or more of these distributions into equivalent uniform properties for analysis.

\* Numbers in brackets refer to references listed in Appendix F.



treatment by operational calculus, e.g. [9, 10, 35, 37] . However, it is evident that a reasonably accurate method of solution which is unrestricted in its application, i. e., can treat completely general distributions of structural characteristics, has advantages over any isolated analytical solution, however rigorous. Thus we shall consider the iterative methods, both graphical and tabular.

Stodola's work on turbines [8] suggested treating non-uniform beams by integrating Rayleigh's differential equation numerically or graphically. His methods also included the use of the funicular polygon, and Rayleigh's rule of minimum frequency. In using the successive integration method, he observed that while the two-noded mode behaved well, a three-noded starting assumption diverged during iteration until it returned to the two-noded mode; he failed to recognize the mutual orthogonality of normal modes. Vianello introduced this method into general use in the field of engineering, and J. L. Taylor and Schadlofsky applied it to ships (illustrated in [68] , as is Lewis's tabular method). All worked primarily with the first, or two-noded mode, and employed the simple-bending form of the equation of Rayleigh, though Taylor recognized the importance of Timoshenko's shear term in ship vibrations. Koch [2] proved the convergence of the method, and more important, demonstrated the necessity of rigorously maintaining orthogonality with all previous modes throughout the iterative method.

Murray demonstrated the importance of transverse shear in ship vibrations [70], and introduced this effect into Schadlofsky's method, (rather than applying it as a subsequent correction to frequency as did McGoldrick [68]). This refined method, first employed by Murray and the authors in work on a submerged submarine [71], was noted also for a method of determining the slope axis by integration by parts of the

treatment by operational methods, e.g. [7, 10, 12, 17]. However, it is evident that a reasonably accurate method of solution which is not affected by the application of the method, i.e., the most completely general distribution of structural characteristics, has advantages over any isolated analysis solution, however rigorous. For we shall consider the iterative methods, both graphical and numerical.

Stodola's work on iterative [1] suggested treating non-uniform beams by integrating Timoshenko's differential equation numerically or graphically. His methods also included the use of the finite difference method, and his iterative method of numerical integration. In using the successive integration method, he observed that while the two-noded mode behaved well, a three-noded starting assumption diverged during iteration until it returned to the two-noded mode, he called it two-noded mode (which is not a nodal mode). Vlasov introduced this method into general use in the field of engineering, and J. E. Taylor and Karandikar applied it to ships (illustrated in [2]), as it is called (which is not a nodal mode). All worked previously with the first, or two-noded mode, and treated the ship's bending form of the equation of bending. Though Taylor recognized the importance of Timoshenko's shear term in his iteration. Kohn [3] turned the convergence of the method, and more important, demonstrated the necessity of rigorously maintaining orthogonality with all previous modes throughout the iterative method.

Many demonstrated the importance of Timoshenko's shear in ship vibrations [4], and introduced the effect into Stodola's method (which then applying it as a convergent correction to the first mode and the second mode). This method is now, first suggested by Timoshenko and the authors in work on a two-noded mode, [5], was used also for a method of determining the slope and deflection by means of the



expression for orthogonality with rigid body rotation (pitch and yaw), and for calculation of higher modes than the first, facilitated by application of Koch's intermode orthogonality conditions.

Other graphical methods have been employed by Southwell, Gumbel, Morrow, and others, but were suitable generally for the first mode only.

Recently the principles employed in these iterative methods have been applied to lump parameter formulations for solution by sequence controlled digital calculators or by electrical analogy. [56, 67, 69]. In particular, Prohl and Myklestad developed finite difference equations for use in the Taylor Model Basin digital computer, taking account of simple bending, shear deflection, and rotatory inertia. Calculations made by this method for vibration in the vertical plane for the attack transport U.S.S. NIAGARA with and without the secondary effects (shear flexibility and rotatory inertia) were compared with experimental data. The increasing importance of secondary effects with increasing frequency was demonstrated, but for this vessel rotatory inertia was relatively unimportant in the vertical plane.

## B. Objective

This investigation is concerned specifically with transverse vibration in the horizontal plane of a submerged submarine. Recent interest in the bodily response of a submarine to dynamic loading such as that accompanying a non-contact underwater explosion has led to the development of methods of modal analysis. These methods, and other demands of modern ship design, have underscored the inadequacy of older established methods of determining natural modes of response of a ship.

In particular, then, the object of this investigation was to establish and demonstrate a method of determining, as accurately as possible, the curvature pattern, deflection pattern, and natural frequencies of the normal





nodes of interest in a transversely vibrating submarine of general mass, inertia, and section area distribution. Further, the method was to be capable of execution without resort to such expensive mechanisms as digital computers.

### C. Method:

The method arrived at is basically similar to that of Schadlofsky [68], involving iterative cycles operating on an assumed deflection pattern, each cycle comprising the four successive integrations suggested by Rayleigh's equation. The Timoshenko shear deflection correction is made to the slope obtained from bending, and the rotatory inertia correction of Rayleigh is made to the derivative of the moment. Koch's orthogonality checks are made to filter out components of all lower modes, and Murray's slope axis determination is employed. Integration is performed by integraph (Plate I), and calculations by commercial desk calculator. The method is demonstrated for the first three normal modes, using structural data from an existing three-eighths scale model of a submarine, including an allowance for entrained water.

Shear lag, and the possible interaction between shear deflection and rotatory inertia are not considered.

It is noted that the problem is formulated as an eigenvalue problem, in which solutions exist only for a discrete set of squared frequencies, each of which corresponds to a particular mode pattern, except that the zero eigenvalue indicates both translational and rotational vibration of the rigid body.



model of interest is a linearly invariant summation of general terms, local, and section area distribution. Further, the method was to be capable of association without need for such expensive mechanisms as digital computers.

### C. Method:

The method involved at its basic is similar to that of [1], involving iterative cyclic operations on a network of elements. Each time completing the four successive iterations suggested by the algorithm's operation. The time taken for a complete iteration is made to be the same as the time taken for the previous iteration. From an analysis it was found that the derivative of the network's output function with respect to the input was made to be the same as the derivative of the input function. The method is demonstrated for the first three network models, giving structural data from an existing three-stage network model of a summation, including an allowance for network delay.

First, let  $y$  and  $x$  be the network's input and output respectively, and  $z$  be the network's output. It is noted that the problem is formulated as an algebraic problem, in which solutions exist only for a discrete set of applied inputs, each of which corresponds to a particular model pattern. Except that the two arguments indicated by functional and relational notation in the right side.



PLATE I  
CORADI INTEGRAPH (28" Beam)  
SHOWING ARGUMENT AND INTEGRAL CURVES



## II PROCEDURE

The basic theory of vibration of a non-uniform, transversely vibrating beam as formulated by Lord Rayleigh, considering only the simple bending effect, can be expressed briefly by:

$$-\frac{\partial^2}{\partial x^2} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = -my\omega^2 \quad (1)$$

where the assumption of simple harmonic motion of circular frequency has been incorporated. Four successive integrations with respect to this equation comprise the framework upon which the iterative method of this investigation has been developed. The deflection and frequency included in the inertia loading term on the right side of (1) are assumed values, and the deflection indicated on the left, obtained by the quadruple integration, is the first estimate of the true deflection pattern. The first estimate of the square of the true circular frequency is obtained by comparison of the assumed and derived deflection antinodal amplitudes, i.e.,

$$(\omega^2)_{\text{final}} = \frac{(\omega^2 y^*)_{\text{start}}}{(y^*)_{\text{final}}} \quad (2)$$

Note also that the second integral of inertia loading, which is the curve of bending moment, must be divided by the curve of local flexural rigidity before the last two integrations are performed, since  $EI$  is a variable in  $x$ .

We first modify the process by applying to the shear curve yielded by the first integration a correction for rotatory inertia, as follows:

$$\frac{dM}{dx} = Q - \frac{Im\gamma}{A} \quad (3)$$

where  $\gamma$  is the bending component of slope. Since this quantity occurs later in any iterative cycle than  $M$ , we must use the  $\gamma$  value from the previous cycle as a best approximation. Thus this correction is not made



## II PROCEDURE

The basic theory of vibration of a non-uniform, transversely vibrating beam as formulated by Lord Rayleigh, considering only the simple bending effect, can be expressed briefly by:

$$(1) \quad \frac{d^2}{dx^2} \left( EI \frac{d^2 y}{dx^2} \right) + \rho A \omega^2 y = 0$$

where the assumption of simple harmonic motion of circular frequency has been incorporated. Four successive integrations with respect to this equation comprise the framework upon which the iterative method of this investigation has been developed. The deflection and frequency included in the iterative loading term on the right side of (1) are assumed values, and the deflection obtained on the left, obtained by the quadrature integration, is the next estimate of the true deflection pattern. The first estimate of the square of the true circular frequency is obtained by comparison of the assumed and derived deflection amplitudes, i.e.,

$$(2) \quad \omega^2 = \frac{\int_0^L \omega^2 y^2 dx}{\int_0^L y^2 dx}$$

Note also that the second integral of beam loading, which is the curve in bending moment, must be divided by the curve of beam deflection, slightly before the last two integrations are performed, since  $EI$  is a variable in  $x$ . The first modifies the process by applying to the shear curve yielded by the first integration a correction for rotary inertia, as follows:

$$(3) \quad \frac{dM}{dx} = \rho A \omega^2 y$$

where  $\gamma$  is the bending component of shear. Since this quantity occurs later in any iterative cycle than  $EI$ , we must use the  $\gamma$  value from the previous cycle as a best approximation. Thus this correction is not made

until the second cycle of each mode.

The second modification of the process is the correction of the slope curve obtained by integrating the curvature distribution. This correction introduces the effect of shear flexibility, and the total slope resulting is expressed by:

$$\eta' = \gamma + \beta \quad (4)$$

where

$$\beta = \frac{Q}{KAG} \quad (5)$$

Here the shear value  $Q$  has been obtained before the slope calculation, and the correction can be made in each cycle.

Application of boundary conditions and orthogonality conditions, including orthogonality (with respect to mass) with the zero eigenvalue motions and with all modes below the one being obtained, at the appropriate points in and after each cycle, determine the four constants of integration, and refine the pattern obtained from the iterative cycle.

In this investigation, since mass and flexural and shear rigidities have general, hence irregular distributions, analytical integration was not possible. The process was therefore carried out in each instance by means of a mechanical integrator with a 28" carriage beam (manufactured in Switzerland by G. Coradi). This instrument draws an integral curve when its stylus is made to trace the argument curve (Plate I). All calculations have been carried out and plotted at intervals of one-fortieth of ship length, as this was found to be the greatest interval which would detect important variations in the functions used.

The sequence of operations is as follows:

- (1) The structural characteristics needed are computed from available plans and tabulated (Appendix E).





- (2) A tabulation of functions of structural characteristics and of the centroid of virtual mass which will be used throughout the work is prepared. (Table IV, Appendix C). To this table are added a tabulation of a function of each modal deflection pattern obtained, for use in orthogonalizing subsequent modes.
- (3) A tabulation of constants computed for repeated use is prepared (Table V, Appendix C).
- (4) A tabulation of the Rayleigh mode patterns (obtained from application of his simple-bending theory for a uniform slender beam) is prepared, for all modes to be obtained. These are normalized patterns.
- (5) The Rayleigh pattern for the mode being calculated is orthogonalized with translation, rotation, and all previous modes (there would be none for the two-noded mode). In practice, the Rayleigh pattern need not be used, any curve of the correct general shape is suitable for a starting guess, and the better the guess, the more rapid the conversion to an acceptable solution. Perhaps the best choice would be the pattern from a similar ship, if such is available.

The resulting orthogonalized pattern will now be used as a starting point of the iterative process.

- (6) Inertia loading based on this pattern is computed and plotted as a function of  $X/L$  along the length of the ship.
- (7) This is integrated to obtain shear, to which is added the rotatory inertia term, giving the derivative of bending moment. (In the first cycle of each mode shear is the differential of bending moment).  $M'$  is plotted.



- (2) A tabulation of functions of structural characteristics and of the contents of virtual mass which will be used throughout the work is prepared. (Table IV, Appendix C). In this table are added a description of a function of each model definition pattern obtained for each orthogonalizing and segment model.
- (3) A tabulation of constants computed for segments and is prepared (Table V, Appendix C).
- (4) A tabulation of the Rayleigh mode patterns (omitted from application of the single-segment theory for a uniform slender beam) is prepared. For all models to be analyzed. These are normalized values.
- (5) The Rayleigh pattern for the mode being calculated is obtained from with variation, values, and all previous modes (these would be used for the two-segment mode). In practice, the Rayleigh pattern used is the same for all modes of the current segment except in analysis for a bending mode, and the values for the mode are used for the conversion to an acceptable solution. Between the last choice would be the pattern for a bending mode, it can be available.
- The resulting orthogonalized pattern will now be used as a starting point of the iterative process.
- (6) Finite element based on this pattern is computed and plotted as a function of  $x/L$  along the length of the ship.
- (7) This is repeated to obtain shear, so which is added the primary bending term, giving the derivative of bending moment. The last eight of each mode form is the derivative of bending moment. 96 is plotted.

- (8)  $M'$  is integrated, and the integral curve is divided, at each station, by flexural rigidity to obtain curvature, which is plotted.
- (9) Curvature is integrated to obtain the bending component of slope to within a constant.
- (10) Shear is divided by shear rigidity, and the resulting shear slope is added to the bending component, and the total is used in an auxiliary process to determine a constant which locates the axis (the constant of the integration in (9)), which is added also, to give total slope. This is plotted.
- (11) Slope is integrated to give the deflection pattern to within an additive constant, which is used in an auxiliary process to obtain the axis shift. The resultant deflection pattern is tabulated.
- (12) This pattern is now submitted to the same orthogonality checks used on the starting guess. The corrected curve is normalized to give a pattern whose value is unity at the center - or center-most-after-antinode. The unnormalized value at this antinode is retained for use in the frequency calculation. The normalized pattern becomes the basis of the inertia loading for the next cycle, or is accepted as the final pattern. This decision is based on the amount of change of deflection pattern and frequency between this cycle and the previous one.
- (13) Frequency is obtained by equation (2), where  $(y^*)_{\text{start}} = 1$ , and  $(y^*)_{\text{final}}$  is the unnormalized antinode deflection retained in (12).
- (14) In the final cycle of each mode, the curvature pattern is normalized by dividing throughout by  $(y^*)_{\text{final}}$  and retained as the final curvature pattern.
- (15) The shear slope distribution is subtracted from the corrected total slope distribution to give the corrected bending slope. This

- (6)  $\phi$  is integrated, and the integral curve is divided, at each station, by the total velocity in order to obtain  $\phi$ , which is added.
- (7)  $\phi$  is integrated to obtain the bending component of slope in which  $\phi$  is constant.
- (8)  $\phi$  is divided by  $\phi$  at each station, and the resulting shear slope is added to the bending component, and the total is used in an iterative process to determine a constant which locates the curve (the constant in the integration is  $\phi$ ), which is added also, to give total slope. This is divided.
- (9)  $\phi$  is integrated to give the deflection pattern in which an additive constant, which is used in an iterative process to obtain the curve, is added. The resulting deflection pattern is divided.
- (10) This pattern is now substituted in the same orthogonally shape used on the bending curve. The corrected curve is normalized to give a pattern which is only at the center - of center - mass - axis - axis. The normalized curve at this station is retained for use in the frequency calculation. The normalized pattern becomes the basis of the next iteration.
- (11) This process is repeated for the total pattern. This process is used on the amount of change in deflection, slope and frequency between this cycle and the previous one.
- (12) Frequency is obtained by equation (1), where  $\phi$  is  $\phi$  and  $\phi$  is  $\phi$ .
- (13)  $\phi$  is the unnormalized random deflection pattern in (12).
- (14) In the final cycle of zero mode, the random pattern is normalized by dividing it by  $\phi$  and retaining in the final constant pattern.
- (15) The shear slope distribution is subtracted from the corrected total slope distribution to give the corrected bending slope. This



is normalized by division by  $(y^*)$  final and retained for use in the rotatory inertia correction of the subsequent cycle.

- (16) The pattern obtained in (12) is now used to repeat the entire cycle, if deemed necessary.

Appendix A, Part 1 contains the derivations of the important relations used. Part 2 is a detailed outline of the procedure, containing formulations of all operations as used in the application of the method illustrated in Appendices B and C.

The method was executed using structural data for a three-eighths scale model of a submarine, and three lateral modes were computed, each having been carried through three cycles of iterations, this being sufficient to show convergence of the method and to reduce residual error, as indicated roughly by the last increment of change, to an acceptable level.

is maintained by means of (2) the fact that the  
 man in the vicinity of the collection of the  
 residue.

(6) The fact in (6) is now used to repeat the  
 residue, it is now used to repeat the

Specific 1. The 1 species the definition of the hypothesis relations  
 need. The 2 is a relation within the hypothesis, containing formal  
 of all operations as used in the application of the method illustrated in  
 Specific 1 and 2.

The method was concerned using statistical data for a three-criteria  
 scale based on a number of, and three other criteria were considered, each  
 being now called three types of relations, this being sufficient  
 to show: however in the method used to reduce residual error, we  
 indicated partly by the fact that the method, to an acceptable level,



### III RESULTS

By observation of the convergence of the results of iteration, patterns, normalized to give a value of unity at the center or centermost after antinode, are presented in Figure XXXIII. The corresponding curvature patterns, from which strain patterns can be obtained, were normalized in such a manner that when the factor between normalized deflection and true deflection maxima has been obtained, this same factor multiplied by the normalized curvature pattern will give the maxima of the true curvature pattern, all dimensions being consistent. These patterns are presented in Figure XXXIV.

Table I gives the final frequencies and periods of the three modes calculated for the model:

TABLE I  
NATURAL TRANSVERSE FREQUENCIES AND PERIODS  
FOR 109.5 FOOT SUBMERGED SUBMARINE MODEL

Mode	Natural Freq. (cpm)	Ratio	Period (milliseconds)
1	417	1	143.9
2	839	2.01	71.5
3	1391 *	3.34	43.1

\* See supplementary discussion, Appendix D

Table II lists the offsets of the normalized deflection and curvature patterns pictured in Figures XXXIII and XXXIV.

As to the method developed, it was found to be convergent, though it appears that the delayed introduction of rotatory inertia into the process must act to delay convergence. This is compensated for by the frequent orthogonality checks, especially their application to the starting pattern.

The results and method are discussed in the subsequent section.

# REFERENCES

1. A comparison of the conversion of the results of the

patients, as obtained in the series of work at the present

series of work, was presented in Figure XXIII. The

corresponding curves obtained from the other series are

shown, were obtained in such a manner that when the factor

between converted deflection and the deflection curve has been

obtained, the same factor multiplied by the measured curves between

will give the curves of the true current pattern, as discussed

being consistent. These factors are presented in Figure XXIV.

Table I gives the first frequencies and periods of the three modes

calculated for the model.

TABLE I

TABLE I. FREQUENCY, PERIOD, AND MODE  
FOR THE TWO-DEGREE-OF-FREEDOM MODEL

Mode	Frequency (cps)	Period (msec)
1	117	8.55
2	157	6.37
3	197	5.08

For comparison of the results, Figure II

Table II gives the effects of the measured deflection and conversion

factor obtained in Figures XXIII and XXIV.

As the results obtained, it was found to be consistent, though

it appears that the delayed indication of velocity results from the

case must not be due to delay in the system. This is accounted for by the

frequency of the system, especially when the frequency is in the

resonance region.

The results are shown and discussed in the subsequent section.



TABLE II

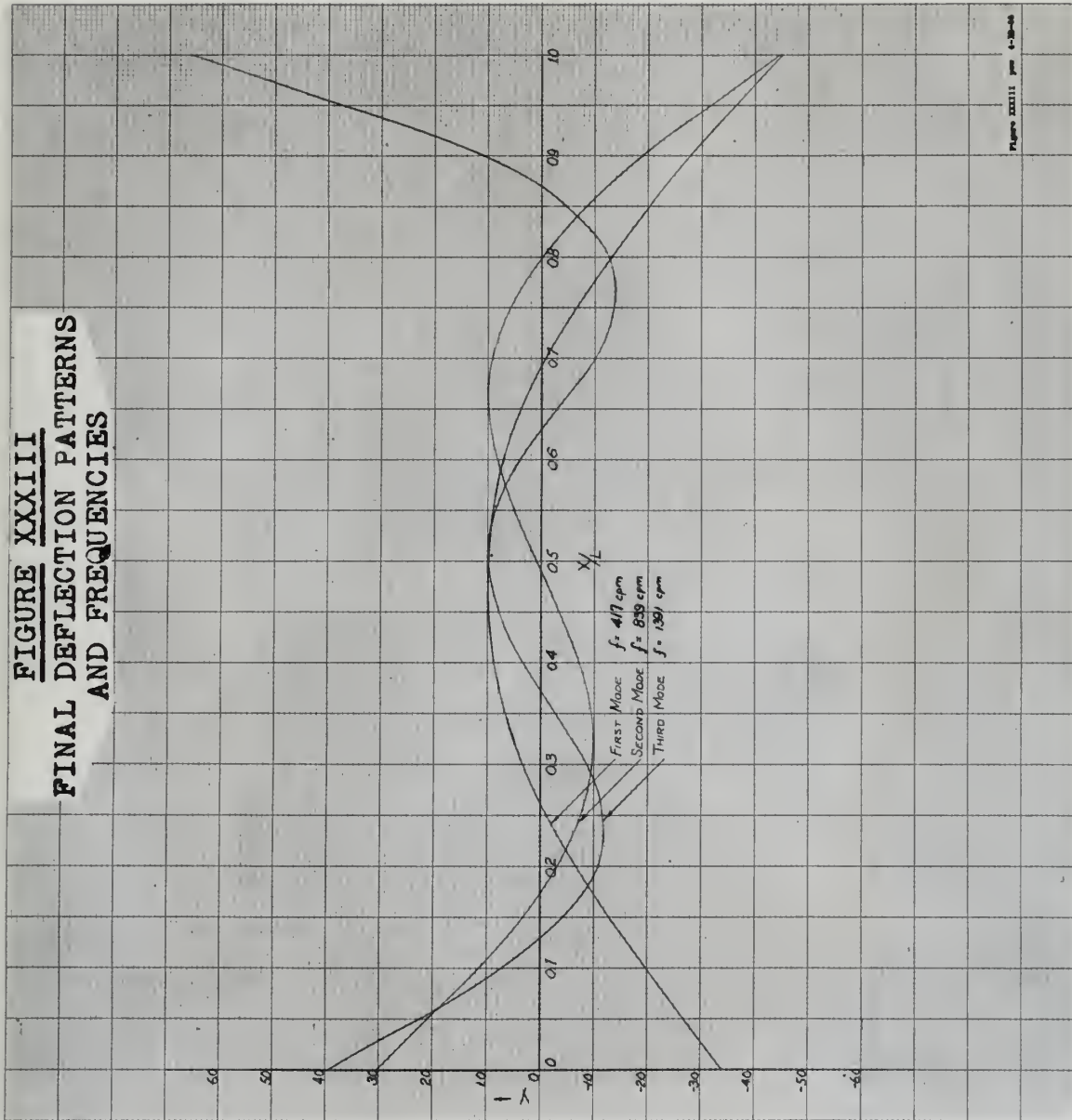
## ORDINATES OF NORMALIZED DEFLECTION AND CURVATURE PATTERNS

$\pi/L$	$y_1$	$y''_1$	$y_2$	$y''_2$	$y_3$	$y''_3$
0.000	-3.408	0	+3.041	0	+3.925	0
0.025	3.054	- 258	2.578	+ 349	3.116	+ 1916
0.050	2.695	947	2.101	2440	2.231	9581
0.075	2.347	1614	1.609	4792	1.430	14371
0.100	1.994	1840	1.161	6142	0.729	15808
0.125	1.641	2117	0.726	6515	+0.102	16968
0.150	1.299	2368	+0.336	6761	-0.417	16727
0.175	0.988	2554	-0.018	6987	0.808	15751
0.200	0.682	2811	0.307	7316	1.074	15005
0.225	0.395	3076	0.567	7506	1.178	13877
0.250	-0.125	3374	0.762	7589	1.158	11536
0.275	+0.109	3644	0.891	7527	1.071	9337
0.300	0.325	4054	0.963	7541	0.862	6815
0.325	0.511	4513	0.977	7474	0.596	3896
0.350	0.661	4921	0.940	7074	-0.295	+ 489
0.375	0.788	5093	0.845	6138	+0.036	- 2801
0.400	0.878	4468	0.706	4352	0.330	4852
0.425	0.944	3240	0.532	2295	0.602	4899
0.450	0.987	2836	0.336	1317	0.793	5445
0.475	1.000	2832	-0.133	+ 588	0.928	5977
0.500	0.989	2842	+0.085	- 154	0.996	6130
0.525	0.965	3361	0.302	1076	0.975	6860
0.550	0.912	4517	0.519	2627	0.875	8160
0.575	0.835	5131	0.693	4400	0.677	7271
0.600	0.711	5019	0.838	5625	0.417	4458
0.625	0.568	4763	0.938	6576	+0.083	- 861
0.650	0.383	4502	0.995	7397	-0.275	+ 3024
0.675	+0.169	4309	0.987	8124	0.655	6922
0.700	-0.087	4018	0.920	8658	1.002	10868
0.725	0.350	3727	0.780	8860	1.227	14653
0.750	0.654	3367	0.582	8808	1.373	17691
0.775	0.976	3045	+0.304	8570	1.389	20157
0.800	1.310	2648	-0.024	7845	1.281	21278
0.825	1.662	2365	0.411	7405	0.951	22813
0.850	2.032	2259	0.849	7406	-0.487	25419
0.875	2.389	2778	1.337	9187	+0.155	34836
0.900	2.783	3341	1.883	10760	1.044	44791
0.925	3.207	2368	2.503	7843	2.297	39530
0.950	3.624	1036	3.174	2091	3.711	21556
0.975	4.066	- 129	3.808	-5228	5.154	+ 1437
1.000	-4.502	0	-4.450	0	+6.590	0

[illegible]



**FIGURE XXXIII**  
**FINAL DEFLECTION PATTERNS**  
**AND FREQUENCIES**







#### IV. DISCUSSION OF RESULTS

##### A. Mode Patterns:

The shape of the mode patterns (Figure XXXIII) generally met the authors' expectations. In ships and submarines, both the mass and flexural rigidity are greater at the midsection than at the ends. This has the effect of altering the mode shape so that the center deflections are smaller relative to the end deflections than in a uniform beam, and the nodal points are closer to the midship section. For example, for the first mode, the center deflection maximum of the uniform beam amounts to 60% of the end deflection, while in the case of the submarine studied, the anti-node deflection was 29.4% of the bow deflection and 22.8% of the stern deflection. The nodes for the uniform beam occur at 22% from the ends, while for the submarine investigated, they occurred at 26.2% from the bow and 30.7% from the stern respectively.

Figure I clearly demonstrates the pronounced increase in mass, sectional area, moment of inertia, and shear factor  $K$  in the vicinity of the hard tank and conning tower amidships in the case of the submarine. One can readily arrive at two consequences of this discontinuity of structural strength. The first of these has already been mentioned - that of the very high end deflections relative to those in the middle body. This behavior will obviously occur for all the modes, with the resulting trend towards patterns with many small loops in the middle with large end branches. It is easy to see how any loading, such as an explosion, occurring in the middle body of a submarine can incur very large deflections at the ends with attendant small deflection near the load. In the case of the third mode, the deflection at the stern will be six times that at the center! The reader will readily appreciate the importance of this characteristic of the mode patterns with respect to the bodily

# IV. DISCUSSION OF RESULTS

## A. Mode Patterns

The shape of the mode patterns (Figure XXIII) generally meet the inherent requirements. In edge and intermediate, both the mass and linearly rigidly are present at the midsection than at the ends. This has the effect of altering the mode shape so that the center deflection is smaller relative to the end deflections than in a uniform beam, and the nodal points are closer to the midship section. For example, for the first mode, the center deflection maximum of the uniform beam amounts to 50% of the end deflection, while in the case of the submarine studied, the anti-node deflection was 57.4% of the end deflection and 51.4% of the stern deflection. The nodes for the uniform beam occur at 21% from the ends, while for the submarine investigated, they occurred at 35.5% from the bow and 36.7% from the stern respectively. Figure I clearly demonstrates the pronounced increase in mass, sectional area, moment of inertia, and shear factor  $K$  in the vicinity of the hard tank and loading tower situated in the case of the submarine. One can readily arrive at two assumptions of this discontinuity of structural strength. The first of these has already been mentioned - that of the very high end deflections relative to those in the middle body. This behavior will obviously occur for all the modes, with the resulting trend towards patterns with many small loops in the middle with large end deflections. It is easy to see how any loading, such as an explosion, occurring in the middle body of a submarine can cause very large deflections at the ends with attendant small deflections near the load. In the case of the third mode, the deflection at the stern will be the same that at the center. The reader will readily appreciate the importance of this characteristic of the mode patterns with respect to the body



response of submarines. Severe whipping may be expected to occur whenever the contributions of the odd and/or even numbered mode end deflections become additive for any general vibratory motion of the submarine!

The second consequence of the irregular distribution of structural characteristics becomes manifest in the effect of rotatory inertia and shear flexibility. For a given structure mass per unit length, a submarine will be many times stiffer than a solid prismatic bar, since in the case of the former, the crosssection is essentially a circular ring of material relatively distant from its centroid. This fact, coupled with that of the nature of the longitudinal distribution of flexural rigidity, explains why the magnitude of the rotatory inertia correction to the frequency of the first mode is somewhat greater for the submarine than for the uniform beam or even a surface ship. Its increased moment of inertia for a given mass, if it is compared to the uniform beam results in larger values of  $\frac{I_m}{A}$ . Even for equal slopes, the rotatory inertia correction would  $\frac{I_m}{A}$  be larger for the submarine. However, the increased flexibility of the end sections over their uniform beam counterparts causes the slope to increase over all the length of the submarine, the greatest increase occurring between the ends and the nodes. The steepness of all the deflection patterns near the ends results in a rather selective effect of rotatory inertia upon the patterns. In general, the effect of rotatory inertia upon the mode patterns studied was most pronounced at the ends, and resulted in an increase in the relative deflection. A comparison of experimental and theoretical mode patterns for a similar submarine where rotatory inertia was neglected in the latter calculation, shows larger experimental end deflections than the theory predicted. The authors feel that such a discrepancy may be due to the application

response of the system. In the case of the system, the response may be expected to be linear  
 whenever the excitation is of the odd and/or even numbered modes and  
 deflection becomes additive for any general vibratory motion of the  
 system.

The second consequence of the principle of distribution of structural  
 characteristics becomes manifest in the effect of rotary inertia and  
 shear flexibility. For a given structure mass per unit length, a shear  
 modulus will be a given value whether there is a solid rectangular bar, a plate  
 in the case of the beam, the excitation is essentially a sinusoidal wave  
 of material property distinct from the control. This fact, coupled with  
 that of the nature of the mathematical distribution of flexural rigidity,  
 requires that the magnitude of the rotary inertia correction in the  
 frequency of the first mode is somewhat greater for the aluminum than  
 for the ordinary beam or even a uniform wire. The increased moment of  
 inertia for a given mass is compared to the uniform beam results in  
 larger values of  $\omega$ . For the equal slopes, the rotary inertia  
 correction would be larger for the aluminum. However, the in-  
 creased flexibility of the end sections over their uniform beam counter-  
 parts causes the slope to increase over all the length of the aluminum.  
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 shows large experimental and theoretical that the theory predicted.  
 The evidence that such a discrepancy may be due to the application



of an incomplete theory, particularly in the light of the results of this thesis.

Shear flexibility apparently causes little change in the shape and magnitudes of the normalized mode patterns. The shape of the KAG curve (Figure I) indicates that the location of maximum shear near the quarter points of the submarine will produce the largest value of shear slope. Actually the percent contribution to  $y'$  of shear slope was much more uniform over the full length of the submarine than was expected. This accounts for the small effect of shear deflection on the normalized patterns. As was expected, the value of  $\beta$  increases with increasing modes numbers, and thus the effect of shear deflection upon frequency becomes more important with increasing mode number. Frequency behavior is discussed more fully in a subsequent section. It should be mentioned, however, that with the third mode, the percentage of shear component of slope gave indications of becoming larger in the region of the middle body, where the greatest effect on  $\omega^2$  will result.

#### B. Curvature Patterns:

The curvature patterns, Figure XXXIV, clearly indicate possible regions of structural weakness with respect to resistance to damage from whipping. In particular, it appears that excessive strain can be expected at about  $X/L = 0.9$  in the event of third mode whipping of significant magnitude\*. The magnitude of the strain wire, of course, also depends upon the radius of the submarine hull at this location.

\* Note that the rudder, propellers and shafting add mass with little attendant increase in stiffness in this part of the submarine.

of an isotropic theory, particularly in the light of the results of this  
 study.

Shear flexibility apparently causes little change in the shape and  
 magnitude of the notched mode patterns. The shape of the KAS  
 curve (Figure 1) indicates that the location of maximum shear near  
 the quarter points of the specimen will produce the largest value of  
 shear stress. Actually the percent contribution to  $\gamma'$  of shear stress  
 was much more uniform over the full length of the specimen than was  
 expected. This accounts for the small effect of shear deflection on  
 the notched pattern. As was expected, the value of  $\gamma'$  increases  
 with increasing mode number, and thus the effect of shear deflection  
 upon frequency becomes more important with increasing mode number.  
 Frequency behavior is discussed more fully in a subsequent section.  
 It should be mentioned, however, that with the third mode, the percent-  
 age of shear component of strain gave indications of becoming larger  
 in the region of the middle hole, where the greatest effect on  $\omega'$  will  
 result.

### 4. Curvature Pattern

The curvature pattern, Figure XXIV, clearly indicates possible  
 regions of structural weakness with respect to resistance to damage from  
 whipping. In particular, it shows that excessive strain can be expected  
 at about  $X/L = 0.5$  in the event of third mode whipping of significant  
 magnitude. The magnitude of the strain rate, of course, also depends  
 upon the value of the resonance half at this location.

\* Note that the rubber, polyurethane and epoxy and resin with their  
 attendant increase in stiffness in this part of the specimen.



Except for the apparent deficiency just noted, the structural design of the submarine appears to be quite well balanced. The general shape of the curvature patterns obtained by the authors agrees quite favorably with experimentally determined curvature patterns of similar submarines.

### C. Frequencies :

Frequency behavior observed in this investigation generally confirmed expectations based on theoretical consideration of the secondary elastic effects. Exact breakdown of these effects is not available in that no complete iterative calculation was conducted without these effects. Although the first cycle of each mode involved no rotatory inertia effect, it cannot be considered as yielding a useful figure for comparisons, because of the predominating effect of the starting estimate curve. However, if we accept these first cycle frequencies for a qualitative examination (Table III, Appendix B), we see the expected drop in frequency when rotatory inertia is introduced, of magnitude greater than what can be attributed to convergence alone. It further appears to be of increasing importance with higher modes.

Data showing the independent effect of shear flexibility is not available, but preliminary investigations summarized in reference [71] showed a similar depression in frequency when this effect was introduced, although the effect was separated for the first mode only. This is generally considered to be of increasing importance for higher modes.

The cumulative result of these effects is noted in the fact that experimentally determined natural frequencies correspond more nearly to the series 1, 2, 3, than to the  $3^2$ ,  $5^2$ ,  $7^2$ , series obtained theoretically from a uniform beam without secondary effects. [69] This same tendency is observed in the frequencies obtained in this investigation, which were seen

Results for the repeated distance test noted, the structural design of the cumulative response to be quite well maintained. The general shape of the cumulative response obtained by the subjects agrees quite favorably with experimentally obtained cumulative response patterns of similar conditions.

### C. Frequency

Frequency was not considered in this investigation generally considered experiments based on theoretical considerations of the secondary effects. Most knowledge of these effects is not available in that no cumulative response relation was conducted without these effects. Although the first type of error mode involved no secondary effects, it cannot be considered as yielding a useful figure for comparison, because of the predominating effect of the starting estimate error. However, if we accept these first type frequencies for a qualitative examination (Table III, Appendix B), we see the expected drop in frequency when secondary effects is introduced, of magnitude greater than what can be attributed to convergence alone. It further appears to be of increasing importance with higher modes.

Data showing the independent effect of these frequencies is not available, but preliminary investigations conducted in various [21] showed a similar depression in frequency when this effect was introduced, although the effect was expected for the first mode only. This is generally considered to be of increasing importance for higher modes.

The cumulative results of these effects is noted in the last two paragraphs, determining initial frequency response curves nearly in the series 1, 2, 3, then to the 1<sup>st</sup>, 2<sup>nd</sup>, 3<sup>rd</sup>, 4<sup>th</sup>, 5<sup>th</sup>, series obtained theoretically from a column beam without secondary effects. [22] The same tendency is observed in the frequency values in this investigation, which were seen



in Table I to correspond to the series 1, 2.01, 3.34, or, for contrast, expressed in the second form;  $3^2$ ,  $4.25^2$ ,  $5.2^2$ .

However, comparison with results of earlier computations for the same model, but without the rotatory inertia effect, cast some doubt on the accuracy of the third mode frequency of 1391 cpm, indicating it is perhaps too high. This suspicion is reinforced by the 3.34 value in the above series. The error (probably amounting to less than 4%) is considered to be due to the fact that the intermode orthogonality check as formulated is inexact for other than the trivial modes, and the error becomes of some importance for the third and higher modes. The more correct formulation is developed in Appendix D, and the third mode frequency corrected by its partial application.

#### D. Sources of Error:

Several sources of error arise from fields the detailed consideration of which is beyond the scope of this thesis, but are noted for completeness.

The first of these stem from the information included as Structural Data, found in Appendix B. A submarine, though more regular in design than a surface ship, is a complex structure, and the participation of its components in carrying the loading are more difficult to analyze than simple beam theory suggests. Therefore agreement is hard to find as to which members should be included in calculating moment of inertia, cross sectional area resisting shear, and the shear distribution coefficient K.

Far more controversial is the matter of entrained water; here it is especially difficult to verify a given theory because vibration theory is not sufficiently refined to permit separating out other effects which interact with the manifestations of entrained water.

Both of the above questions have been sidestepped by utilizing data already calculated from plans, based on the assumptions outlined in

in Table I is compared to the series 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 799, 800, 801, 802, 803, 804, 805, 806, 807, 808, 809, 810, 811, 812, 813, 814, 815, 816, 817, 818, 819, 820, 821, 822, 823, 824, 825, 826, 827, 828, 829, 830, 831, 832, 833, 834, 835, 836, 837, 838, 839

Dr. Robert M. Anderson

cross sections) were located about, and the water distribution wells in which changes would be needed in maintaining amount of water almost every day. The water agreement is best to find a components in action. The meeting was more difficult to analyze than then a water act, is a complex structure, and the participation of its Data, found to be possible in a summary, though more regular is desired. The first of the year from the information included as historical of which is beyond the scope of this thesis, but are noted for comparison. Several sources of other water from fields the detailed consideration

It is not surprising that the results of the present study are in general agreement with those of the previous studies. The results of the present study are in general agreement with those of the previous studies. The results of the present study are in general agreement with those of the previous studies.



Appendix E. It is not to be inferred that these matters are insignificant; they are vital to any prediction of vibration, and are the subject of considerable study.

It has been assumed that deflections are small and that the proportional limits have not been exceeded. Standard values of Young's Modulus and Poisson's Ratio have been assumed. Any departure from these assumptions in a ship tested experimentally can be expected to cause discrepancies.

The second group of errors can be traced to the procedure and to its execution.

First, failure to carry the iteration through sufficient cycles leads to an error. The residual error after a given number of cycles will be large if (a) the starting assumption of deflection was grossly in error, or (b) if orthogonality checks were not properly and regularly applied. An error was introduced also by the application of the inexact orthogonality formulation.

Next, the rotatory inertia term, being based necessarily on data from a previous cycle, is slightly in error.

Further, the failure to make this correction in the first cycle results in a discrepancy which will take several cycles to eliminate entirely.

Shear lag, and probably other lesser effects, have not been considered.

Integrating operation involves some small error. This is periodically filtered out by boundary conditions and other checks, but some residue necessarily remains. In particular, a small amount of lost motion was noted in the integrator used.

Any error in developing a given mode pattern will result in a small error in each subsequent pattern, through the intermode orthogonality checks, which necessarily employ the ordinates of previously determined modes.

Appendix 2. It is not to be inferred that these methods are infallible; they are vital to any prediction of vibration, and are the subject of considerable study.

It has been assumed that deflections are small and that the proportional limits have not been exceeded. Standard values of Young's modulus and Poisson's Ratio have been assumed. Any departure from these assumptions in a ship tested experimentally can be expected to cause discrepancies.

The second group of errors can be traced to the procedure and to its execution.

First, failure to carry the vibration through sufficient cycles leads to an error. The residual error after a given number of cycles will be large if (a) the starting assumption of deflection was grossly in error, or (b) if  $\log_{10} \frac{1}{\delta}$  cycles were not properly and regularly applied. An error was introduced also by the application of the incorrect logarithmic formula.

Next, the wrong number of cycles being used necessarily on data from a previous cycle is highly in error.

Further, the failure to make this correction in the first cycle results in a discrepancy which will make several cycles of difference eventually. Other factors and probably other tested effects have not been considered.

Integrating, however, involves some small errors. This is particularly illustrated by boundary conditions and other checks, but some residue necessarily remains. In addition, a small amount of loss of motion was noted in the integrator used.

Any error is shown by a given mode which will result in a small error in each subsequent pattern through the methods of integration checks, which necessarily require the reduction of measured data to modal.



Finally, the usual remarks concerning human error in computing values, and in plotting, integrating, and reading curves. It is noted, however, that the method by nature tends to reject such error or to make them immediately obvious as the technique becomes familiar.

#### E. Applicability:

The method of this paper is not envisaged as replacing the rather advanced techniques of digital computers and electrical analogs. It may well complement them to some degree, but is intended primarily to serve the needs of the smaller research facility which, operating on a limited budget, is faced with the necessity of performing such determinations with simpler tools. Integragraphs of the type used are found in a number of educational institutions, design offices, and shipyards, but the authors have reason to believe that more than a few of them lie neglected beneath at least a decade's accumulation of dust. The only other tool needed is an automatic or semi-automatic commercial desk calculator. It is considered that the work can be set up on appropriate forms in such a manner that a mathematics aid or junior research assistant can carry it out without special training and under only a modicum of supervision.

Finally, the same remarks concerning human error in computing values, and in plotting, integrating, and reading curves. It is noted, however, that the method by means of which the error is made from immediately visible as the technique becomes familiar.

## II. Application

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special training and under only a condition of supervision.

## V. CONCLUSIONS

Convergence of the iterative method, with secondary effects, as developed in this thesis appears satisfactory, and any tendency towards instability is held in check by periodic application of orthogonality conditions. The method inherently detects and filters out errors, so that a mistake in an early cycle does not invalidate subsequent cycles of the same mode, but may delay convergence slightly.

While the procedure appears complex, it can be programmed on standard forms for application by research assistants without special knowledge of vibrations or advanced mathematics. Equipment required is standard to many installations and inexpensive relative to such special equipment as digital computers.

Results of this method appear to be acceptable; the third mode frequency is considered accurate to within 2.6% after three iterative cycles, while earlier modes, having attained better convergence in the same number of modes, are considerably more accurate.

The secondary effects are considered to have been adequately accounted for, with little added complication to the process. In particular, rotatory inertia appears to have had a somewhat greater effect than has been evidenced in studies of uniform beams; end deflections are seen to be increased relative to antinodal excursions when the effect is considered. Secondary effects thus are included not only in frequency determination, but also in deflection and curvature patterns.

A clear indication of highly loaded sections is presented by the normalized curvature distributions which result directly from the process. Structural inadequacies are thus pointed out.

It thus appears that the method developed is well suited to the purposes of small facilities conducting vibration research on limited budgets, where accurate theoretical determination of information on natural modes is needed.



# V. CONCLUSIONS

Governing of the iterative method, with secondary effects, developed in this paper appears satisfactory, and any tendency towards instability is held in check by periodic application of orthogonal conditions. The method tolerantly detects and corrects out errors, as that a mistake in an early cycle does not invalidate subsequent cycles at the same mode, but may delay convergence slightly.

While the procedure appears complex, it can be programmed on standard forms for application by research assistants without special knowledge of vibrations or advanced mathematics. Equipment required is restricted to many installations and inexpensive relative to such special equipment as digital computers.

Results of this method appear to be acceptable. The third mode frequency is considered accurate to within 5% after three iterations, while higher modes, having obtained better convergence in the same number of modes, are considerably more accurate.

The secondary effects are considered to have been adequately accounted for, with little added complication in the process. In particular, rotary inertia appears to have had a somewhat greater effect than has been evidenced in studies of uniform beams; and deflections are seen to be increased relative to unimodal excitation when the effect is considered. Secondary effects thus are included not only in frequency determination, but also in deflection and curvature patterns.

A clear indication of highly loaded sections is presented by the normalised curvature distributions which result directly from the process. Structural inadequacies are thus pointed out.

It thus appears that the method developed is well suited to the purpose of small facilities conducting vibration research on limited budgets, where accurate theoretical determination of information on natural modes is needed.



## VI RECOMMENDATIONS

It is recommended that the method for modal calculations which is described in this thesis be considered by small research activities for similar work. As a general rule, such activities operate under a rather limited budget with correspondingly small allocations for computing equipment of the analog or digital types. On the other hand, most of these activities will certainly own more than one desk calculator and will have the funds available for the purchase of an integrator in the event that one is not owned. At least for the modes of interest, the method used by the authors appears more than satisfactory from the standpoint of moderate equipment costs, simplicity, and availability. Digital computers are in ever increasing demand, and the delay involved in obtaining results from a central calculating contractor may be unacceptable for the small research organization. The method is clearly not competitive with that of the digital computer when the latter is readily available and a large number of mode patterns is desired.

Experimental corroboration of the authors' results would be highly desirable before the method is accepted for general application. For any given submarine design this can be accomplished by mechanical excitation of a model of the proposed vessel, provided it is of sufficiently large scale to assure structural similarity.

It is further recommended that the effect of rotatory inertia as well as shear deflection be included in the natural mode calculations regardless of the method used.

In any future application of this method, it is recommended that the more exact orthogonality conditions which arise from the inclusion of rotatory inertia effects be applied. While it is more convenient to neglect them, such practice may result in the loss of refinements, especially in the higher modes, which are of the same order of magnitude as

## VI. RECOMMENDATIONS

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In any future application of this method, it is recommended that the most exact trigonometric functions be used when the function of memory inertia must be applied. While it is more convenient to neglect them, such neglect may result in the loss of refinement, especially in the higher modes, which are of the same order of magnitude as

the rotatory inertia corrections themselves.

The effect of shear lag in ship-type structures upon their vibration behavior should be investigated.

the relatively inactive corrections themselves.

The effect of short lag in this type of correction upon their vibration

behavior should be investigated.



VII APPENDIX

### APPENDIX

# APPENDIX A

## PART I

### Theoretical Basis:

The submarine is initially considered to behave as a long, thin beam with vanishing shear force and bending moment. Under this assumption, then, the classical theory of the lateral vibration of a free-free non-uniform bar applies. [6],[9]. We are concerned here with the normal modes of vibration. When a bar vibrates in one of its normal modes, the deflection at any location varies harmonically with time. If damping is neglected, the lateral displacement of the bar can be represented by:

$$y = X ( A \cos \omega t + B \sin \omega t ) \quad (6)$$

where  $X$  is a function of the coordinate  $x$ , and determines the shape or pattern of the normal or natural mode of vibration which we are considering.

$X$  is known in the literature as a "normal" or "natural" function.

It is obvious that any particular normal mode of vibration is characterized solely by  $X_i$  and the frequency of that mode,  $\omega_i$ . By superimposing all possible normal modes of vibration, the general expression for the free lateral vibration becomes:

$$y = \sum_{i=1}^{i=\infty} X_i ( A_i \cos \omega_i t + B_i \sin \omega_i t ) \quad (7)$$

In order to calculate a normal mode pattern, one must resort to beam flexure theory. The simple (Euler) theory of bending may be assumed to hold if the vibration occurs in one of the principal planes of flexure of the bar, and if the crosssectional dimensions of the bar are small compared to its length. In the actual case of a ship or submarine the first of these restrictions offers no problem, but the second can be unrealistic, particularly for a modern submarine with its "cigar" form and essentially circular sections. Strictly speaking then, the Euler bending theory is not applicable to a

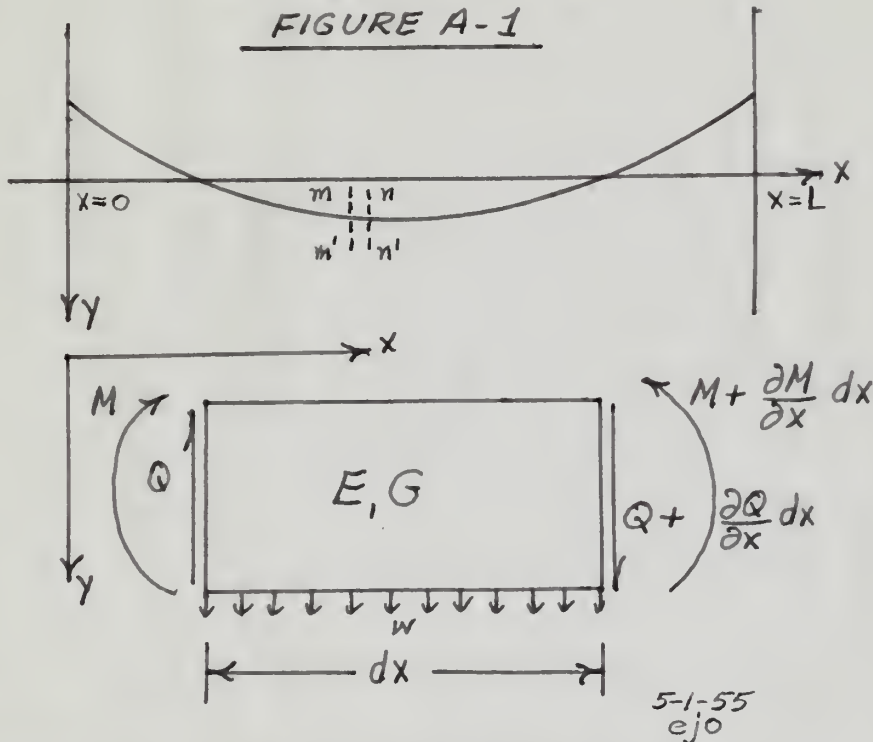




submarine or even a beam whose cross sectional dimensions are appreciable. For the present, however, we shall use the Euler theory as a beginning and modify it subsequently by a correction for shearing force and rotatory inertia effects.

We refer now to Figure A-1, which is a representation of a non-uniform beam and an element thereof. The symbols are in accordance with the Nomenclature, except that  $w$  here is taken as some loading in lb/ft. With the axis oriented as shown, the well known Euler differential equation for the deflection curve is:

$$EI \frac{\partial^2 y}{\partial x^2} = -M. \quad (8)$$



We differentiate (8) once to obtain the shearing force:

$$-\frac{\partial}{\partial x} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = -\frac{\partial M}{\partial x} = -Q. \quad (9)$$

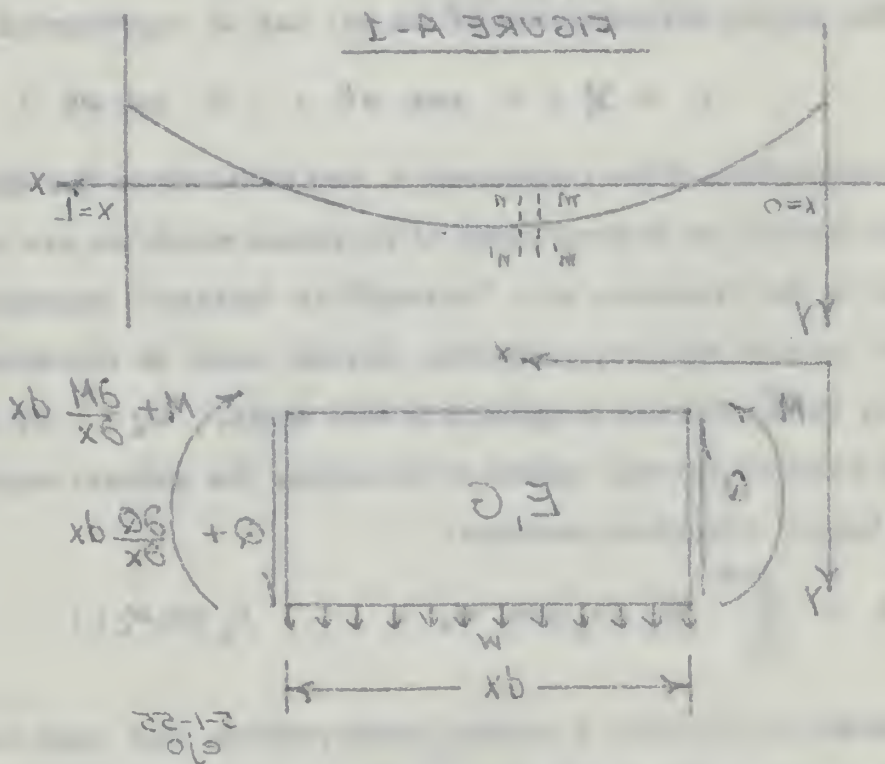
An additional differentiation yields the loading.

$$-\frac{\partial^2}{\partial x^2} \left( EI \cdot \frac{\partial^2 y}{\partial x^2} \right) = -\frac{\partial Q}{\partial x} = w. \quad (10)$$

assumes or even a beam whose deflection is assumed to be appreciable. For the present, however, we shall use the Euler theory as a beginning and modify it subsequently by a correction for shearing force and rotary inertia effects.

We refer now to Figure A-1, which is a representation of a non-uniform beam and an external load. The symbols are in accordance with the Nomenclature, except that  $w$  here is taken as some loading so to  $w$ . With the data assumed as shown, the well known Euler differential equation for the deflection curve is:

$$(1) \quad EI \frac{d^4 y}{dx^4} = w$$



We differentiate (1) once to obtain the shearing force:

$$(2) \quad \frac{d}{dx} \left( EI \frac{d^3 y}{dx^3} \right) = -w$$

An additional differentiation yields the loading:

$$(3) \quad \frac{d^2}{dx^2} \left( EI \frac{d^2 y}{dx^2} \right) = -w$$

Equation (10) is the differential equation of a bar subjected to a continuous load of intensity  $w$ . In the case of free lateral vibration, one can apply D'Alembert's principle and replace  $w$  by the inertia loading

$$- \frac{m}{\partial t^2} \frac{\partial^2 y}{\partial x^2} \quad (11)$$

Substituting (11) for  $w$  in equation (10), the general equation for the lateral vibration of the bar becomes

$$\frac{\partial^2}{\partial x^2} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = - m \frac{\partial^2 y}{\partial t^2} \quad (12)$$

$$\text{Finally, since, for simple harmonic motion, } \frac{\partial^2 y}{\partial t^2} = - y \omega^2 \quad (13)$$

we may write equation (12) as follows:

$$\frac{\partial^2}{\partial x^2} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = m \omega^2 y \quad (14)$$

We now return to the reality that the dimensions of the cross section are not small with respect to the length of the bar. Investigation of the effect which the cross sectional dimensions can have on the frequency are encompassed in the classical literature, [6][9][55]. Of these perhaps the most widely accepted is the treatment of the effects of rotatory inertia and shearing force by Timoshenko in reference [9][55]. There are doubtless other effects introduced by the cross sectional dimensions and configuration but for the purposes of this thesis the Timoshenko mechanism was adopted because it accounts for the two major effects mentioned above.

The effects of both rotatory inertia and shear deflection become of considerable importance in the higher modes of vibration where the bar is subdivided into shorter segments by an ever increasing number of nodes. [9][69].



Equation (6) is the differential equation of a bar subjected to a continuous load of intensity  $W$ . In the case of free lateral vibration, we can apply D'Alembert's principle and replace  $W$  by the inertia load

$$(11) \quad \frac{W}{g} \frac{\partial^2 y}{\partial t^2}$$

Substituting (11) for  $W$  in equation (6), the general equation for the lateral vibration of the bar becomes

$$(12) \quad \frac{\partial^4 y}{\partial x^4} + m \frac{\partial^2 y}{\partial t^2} = 0$$

Finally, since the simple harmonic motion  $y = y \sin pt$ , we may write equation (12) as follows:

$$(13) \quad \frac{\partial^4 y}{\partial x^4} + m \omega^2 y = 0$$

We now return to the reality that the dimensions of the mass and length are small with respect to the length of the bar. Investigation of the effect upon the cross-sectional dimensions has shown that frequency are independent in the classical (bimaterial) case. Of these perhaps the most widely accepted is the treatment of the effects of rotary inertia and shearing force in Timoshenko's reference [7] [8] [9]. These are discussed in other references by the same author, dimensions and configurations for the purpose of this thesis the Timoshenko mechanism was adopted because it accounts for the two major effects mentioned above.

The effects of both rotary inertia and shear deformation become of considerable importance in the higher modes of vibration where the bar is subdivided into shorter segments by an increasing number of nodes.



a. Rotatory Inertia :

Refer again to figure A-1. One can easily see how the element  $dx$  during vibration not only performs a translatory motion, but can rotate as well. The variable angle of rotation is equal to the slope of the deflection curve,  $\partial y / \partial x$ , and the corresponding angular velocity and angular acceleration will be given by

$$\frac{\partial^2 y}{\partial x \partial t}, \text{ and } \frac{\partial^3 y}{\partial x \partial t^2}.$$

The moment of the inertia forces of the element about the axis through its centroid perpendicular to the x-y plane will evidently be:

$$dM_r = - \left( I + \frac{\partial I}{\partial x} \frac{dx}{2} \right) \left( \frac{m}{A} + \frac{\partial(m/A)}{\partial x} \frac{dx}{2} \right) \left( \frac{\partial^3 y}{\partial x \partial t^2} \right) dx \quad (15)$$

(The moment is taken positive when it is clockwise), where  $m/A$  is the mass per unit volume of the non-uniform bar.

We now expand (15), and obtain:

$$\begin{aligned} dM_r = & - I m/A \left( \frac{\partial^3 y}{\partial x \partial t^2} \right) dx - (dx)^2 \left( \frac{\partial I}{\partial x} \frac{m}{2A} + \frac{I}{2} \frac{\partial}{\partial x} (m/A) \right. \\ & \left. + \frac{\partial I}{\partial x} \cdot \frac{\partial(m/A)}{\partial x} \frac{1}{(2)^2} \right) \frac{\partial^3 y}{\partial x \partial t^2} \end{aligned} \quad (15 a)$$

If we neglect second order and higher infinitesimals,

$$dM_r = \left( - \frac{I m}{A} \frac{\partial^3 y}{\partial x \partial t^2} \right) dx \quad (15 b)$$

Remembering again that for simple harmonic motion  $\frac{\partial^2 y}{\partial t^2} = -\omega^2 y$ , we finally obtain:

$$dM_r = - \frac{I m}{A} \frac{\partial}{\partial x} (-\omega^2 y) dx = \frac{I m \omega^2}{A} \frac{\partial y}{\partial x} dx \quad (16)$$

This moment should be accounted for in considering the variation of bending moment along the axis of the bar. Thus, instead of equation (4) we will have:

$$\frac{dM}{dx} = Q + \frac{I m \omega^2}{A} x \frac{\partial y}{\partial x} \quad (17)$$

## A. Rotatory Inertia

Let us return to Figure A-1. You can easily see how the element of bending rotation not only performs a translatory motion, but can rotate as well. The variable angle of rotation is equal to the slope of the deflection curve,  $\theta$ , and the corresponding angular velocity and angular acceleration will be given by

$$\dot{\theta} = \frac{dy}{dx} \quad \text{and} \quad \ddot{\theta} = \frac{d^2y}{dx^2}$$

The moment of the inertia forces of the element about the axis

through its centroid perpendicular to the  $x-y$  plane will evidently be:

$$dM_z = \left( 1 + \frac{y^2}{x^2} \right) \left( \frac{dx}{2} \right) \left( \frac{dy}{2} \right) \left( \frac{d^2y}{dx^2} \right) \quad (1)$$

(The moment is taken positive when it is clockwise, where  $m$  is the mass

per unit volume and  $dx$  and  $dy$  are

We now regard (1), and obtain

$$dM_z = \left( 1 + \frac{y^2}{x^2} \right) \left( \frac{dx}{2} \right) \left( \frac{dy}{2} \right) \left( \frac{d^2y}{dx^2} \right) + \frac{y}{x} \left( \frac{dy}{2} \right) \left( \frac{d^2y}{dx^2} \right) \quad (2)$$

If we neglect second order and higher infinitesimals,

$$dM_z = \left( \frac{1}{2} + \frac{y}{x} \right) \left( \frac{dy}{2} \right) \left( \frac{d^2y}{dx^2} \right) \quad (3)$$

Remembering again that the kinetic energy is  $\frac{1}{2} M \dot{\theta}^2$ , we

finally obtain:

$$dM_z = \left( \frac{1}{2} + \frac{y}{x} \right) \left( \frac{dy}{2} \right) \left( \frac{d^2y}{dx^2} \right) \quad (4)$$

This moment should be included in computing the variation

of bending moment along the axis of the bar. Thus, instead of equation (4)

we will have:

$$\frac{dM}{dx} = \frac{1}{2} \omega^2 \frac{d^2y}{dx^2} + \frac{y}{x} \quad (5)$$

Substituting this in the equation for the deflection curve,

$$\frac{\partial^2}{\partial x^2} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = \frac{\partial M}{\partial x} \quad (18)$$

and using expression (11) and (12), we finally obtain:

$$-\frac{\partial^2}{\partial x^2} \left( EI \frac{\partial^2 y}{\partial x^2} \right) = m \omega^2 y - \omega^2 \frac{\partial}{\partial x} \left( \frac{Im}{A} \frac{\partial y}{\partial x} \right) \quad (19)$$

#### b. Effect of Shearing Force.

A still more accurate differential equation is obtained if not only rotatory inertia but also shear deflection is considered. The slope of the deflection curve depends not only upon the rotation of cross sections of the bar but on the deflection of the sections due to shear as well. Let  $\gamma$  denote the slope of the bar when shearing force is neglected and  $\beta$  the angle of shear at the neutral axis of the same cross section. Evidently the total slope must be the sum of  $\gamma$  and  $\beta$ , i. e.,

$$\frac{dy}{dx} = \gamma + \beta \quad [9] \quad (20)$$

Using the elementary bending theory, we have for bending moment and shearing force the following expressions,

$$M = -EI \frac{d\gamma}{dx}, \quad Q = KAG\beta = K \left( \frac{dy}{dx} - \gamma \right) AG \quad (21) \text{ \& } (22)$$

$K$  is a numerical factor which is a function of the shape of the cross sectional area, usually defined as follows:

$$K = \frac{\text{average shear stress in section}}{\text{maximum shear stress in section}} \quad (23)$$

This is the static shear stress distribution factor used by Timoshenko and discussed in standard texts.







The complete mechanism of the bar including both rotatory inertia and shear deflection effects may now be formulated. Referring to Figure A-1 the differential equation of rotation of the element  $mm_1$  will be:

$$-\frac{\partial M}{\partial x} dx + Q dx = \frac{Im}{A} \cdot \frac{\partial^2 \gamma}{\partial t^2} dx \quad (24)$$

Substituting equations (21) and (22) we obtain:

$$-\frac{\partial}{\partial x} \left( EI \frac{\partial \gamma}{\partial x} \right) + K \left( \frac{\partial y}{\partial x} - \gamma \right) AG - \frac{Im}{A} \frac{\partial^2 \gamma}{\partial t^2} = 0 \quad (25)$$

Neglecting all but first order infinitesimals, the differential equation for the translatory motion of the same element in the horizontal plane will be:

$$\frac{\partial Q}{\partial x} dx = m \frac{\partial^2 y}{\partial t^2} dx, \quad (26)$$

or

$$m \frac{\partial^2 y}{\partial t^2} - \frac{\partial}{\partial x} [KAG \left( \frac{\partial y}{\partial x} - \gamma \right)] = 0. \quad (27)$$

Taken together, equations (25) and (27) comprise the simultaneous differential equations which describe the more complete mechanism of lateral vibration for non-uniform bars.

The complete mechanism of the test including both secondary  
 inertia and shear deformation effects may now be formulated. Referring  
 to Figure 4-1 the differential equation of motion of the element will be

$$(54) \quad \frac{1}{2} \frac{d^2 u}{dt^2} + \frac{1}{2} \frac{d^2 v}{dt^2} = \frac{1}{2} \frac{d^2 w}{dt^2} + \frac{1}{2} \frac{d^2 \theta}{dt^2}$$

Integrating equations (51) and (52) we obtain

$$(55) \quad \frac{1}{2} \frac{d^2 u}{dt^2} + \frac{1}{2} \frac{d^2 v}{dt^2} = \frac{1}{2} \frac{d^2 w}{dt^2} + \frac{1}{2} \frac{d^2 \theta}{dt^2}$$

Integrating all four differential equations, the differential equation  
 for the translational motion of the beam element in the horizontal plane

will be

$$(56) \quad \frac{1}{2} \frac{d^2 u}{dt^2} + \frac{1}{2} \frac{d^2 v}{dt^2} = \frac{1}{2} \frac{d^2 w}{dt^2} + \frac{1}{2} \frac{d^2 \theta}{dt^2}$$

$$(57) \quad \frac{1}{2} \frac{d^2 u}{dt^2} + \frac{1}{2} \frac{d^2 v}{dt^2} = \frac{1}{2} \frac{d^2 w}{dt^2} + \frac{1}{2} \frac{d^2 \theta}{dt^2}$$

Now, taking equations (53) and (54) and (55) and (56) and (57) and  
 differential equations will describe the more complete mechanism of  
 interest vibration for non-linear beam.

$$\frac{\partial}{\partial x} \left( EI \frac{\partial \gamma}{\partial x} \right) + K \left( \frac{\partial y}{\partial x} - \gamma \right) AG - \frac{Im}{A} \frac{\partial^2 \gamma}{\partial t^2} = 0 \quad (25)$$

$$m \frac{\partial^2 y}{\partial t^2} - \frac{\partial}{\partial x} \left[ \left( \frac{\partial y}{\partial x} - \gamma \right) KAG \right] = 0 \quad (27)$$

Timoshenko's analysis of rotatory inertia and shear deflection effects for a uniform bar may be found in reference [9]. He obtains two simultaneous equations similar to (25) and (27).

$$EI \frac{\partial^2 \gamma}{\partial x^2} + K \left( \frac{\partial y}{\partial x} - \gamma \right) AG - \frac{Im}{A} \frac{\partial^2 \gamma}{\partial t^2} = 0 \quad (28)^*$$

$$m \frac{\partial^2 y}{\partial t^2} - K \left( \frac{\partial^2 y}{\partial x^2} - \frac{\partial \gamma}{\partial x} \right) AG = 0 \quad (29)^*$$

He then proceeds to eliminate from equations (28) and (29)  $\gamma$ , and obtains thereby the well known fourth order coupled equation:

$$EI \frac{\partial^4 y}{\partial x^4} + m \frac{\partial^2 y}{\partial t^2} - \left( \frac{mI}{A} + \frac{EI m}{KAG} \right) \frac{\partial^4 y}{\partial x^2 \partial t^2} +$$

$$\frac{m^2 I}{K A^2 G} \times \frac{\partial^4 y}{\partial t^4} = 0 \quad (30)$$

\* Timoshenko's notation has been altered to conform with the authors'.

$$(24) \quad 0 = \frac{8^2 6}{5 \cdot 6} \frac{m}{\Delta} - D \left( \gamma - \frac{1}{6} \right) + \left( \frac{8^2 6}{x 6} \right) \frac{1}{x 6}$$

$$(25) \quad 0 = \left[ D \left( \gamma - \frac{1}{6} \right) \right] \frac{6}{x 6} - \frac{8^2 6}{5 \cdot 6}$$

Timoshenko's analysis of torsion yields the same result

the above is a method for way be found in reference [7]

We obtain two simultaneous equations similar to (22) and (23)

$$(26) \quad 0 = \frac{8^2 6}{5 \cdot 6} \frac{m}{\Delta} - D \left( \gamma - \frac{1}{6} \right) + \left( \frac{8^2 6}{x 6} \right) \frac{1}{x 6}$$

$$(27) \quad 0 = D \left( \frac{8^2 6}{x 6} - \frac{1}{6} \right) - \frac{8^2 6}{5 \cdot 6}$$

The two proceeds to eliminate  $\gamma$  in equations (26) and (27)

and obtain through the well known linear algebraic equation:

$$(28) \quad \frac{8^2 6}{5 \cdot 6} \frac{m}{\Delta} - \left( \frac{8^2 6}{x 6} - \frac{1}{6} \right) = \frac{8^2 6}{5 \cdot 6}$$

$$(29) \quad 0 = \frac{8^2 6}{5 \cdot 6} \frac{m}{\Delta} - \frac{1}{6}$$

\* Timoshenko's analysis can also be used to compare with the above



Equations (25) and (27) appear not to be susceptible to such an operation, since  $I$ ,  $A$ ,  $m$ , and  $K$  are all functions of  $x$  in this case. Considerable effort was spent by the authors in an attempt to eliminate  $\gamma$  from equations (25) and (27), without success. If the operation is possible, and this the authors doubt, the resulting equation will at best be too complex and unwieldy for practical application.

The difficulties involved in the solution of equation (30) are well known. It is quite obvious that the irregular variation of  $I$ ,  $m$ ,  $K$ , and  $A$  with  $x$  in the case of a ship dictates that equations (25) and (27) must be solved in an approximate manner using the graphical, lumped parameter, or Rayleigh-Ritz procedure. The task is so formidable that the authors could find no exact solution in the literature encompassing both shear and rotatory inertia effects applied to an irregular non-uniform bar.

To recapitulate, the basic equations used by the authors are, in simplified form:

$$Q' = -m\omega^2 y \quad (31)$$

$$Q = KAG\beta = KAG(y' - \gamma) \quad (32)$$

$$M' = Q + \frac{Im\omega^2 \gamma}{A} \quad (33)$$

$$\gamma' = \frac{M}{EI} \quad (34)$$

$$y' = \gamma + \beta \quad (35)$$

The boundary conditions applicable to the quadruple integration are:

$$Q = 0 \text{ for } x = 0, \quad x = L \quad (36)$$

$$M = 0 \text{ for } x = 0, \quad x = L \quad (37)$$

operation, about 1.5 m, and all functions of  $x$  in this case.  
 Considerable effort was spent by the authors in an attempt to eliminate  
 $\gamma$  from equations (11) and (12), without success. If the operation is  
 possible, and this has already been done, the following equation will at least  
 be too complex and unwieldy for practical application.

The difficulties involved in the solution of equation (10) are  
 well known. It is quite obvious that the 15-20% variation in  $L$ ,  $\mu$ ,  $\sigma$ ,  $\gamma$ ,  
 and  $\lambda$  with  $X$  in this case of a high dielectric that equations (10) and (11)  
 must be solved in an approximate manner using the graphical method  
 presented in Example 1 in this paper. The task is no formidable  
 but the authors would like to point out that the iterative method  
 also has been used and today's digital devices applied in an iterative manner.

unknown are:  
 To recompute, the same operation using the above are:

in simplified form:

- (31) 
$$\frac{1}{\sigma} = \frac{1}{\sigma_0} - \frac{1}{\sigma_0} \frac{1}{\sigma_0}$$
- (32) 
$$\frac{1}{\mu} = \frac{1}{\mu_0} - \frac{1}{\mu_0} \frac{1}{\mu_0}$$
- (33) 
$$\frac{1}{\gamma} = \frac{1}{\gamma_0} - \frac{1}{\gamma_0} \frac{1}{\gamma_0}$$
- (34) 
$$\frac{1}{\lambda} = \frac{1}{\lambda_0} - \frac{1}{\lambda_0} \frac{1}{\lambda_0}$$
- (35) 
$$\frac{1}{L} = \frac{1}{L_0} - \frac{1}{L_0} \frac{1}{L_0}$$

The constant conditions apply to the dielectric properties are:

- (36) 
$$\frac{1}{\sigma} = \frac{1}{\sigma_0} - \frac{1}{\sigma_0} \frac{1}{\sigma_0}$$
- (37) 
$$\frac{1}{\mu} = \frac{1}{\mu_0} - \frac{1}{\mu_0} \frac{1}{\mu_0}$$

Equations (31) - (37) describe an eigenvalue problem, with  $\omega^2$  being the eigenvalue. Solutions will exist only for a discrete set of  $\omega^2$ . For each such value of  $\omega^2$  a unique mode pattern will correspond. An exception to this is the case when  $\omega^2 = 0$ , where two possible modes exist:  $y = 1$  (rigid body translation), and  $y = 1 - \frac{x}{x}$  (rigid body rotation). The two modes for this unique case have been orthogonalized for convenience.

Useful orthogonality relations exist between the normal mode patterns. In particular, for each

$$\int_0^L m y_i dx = 0 \quad (38)$$

$$\int_0^L x m y_i dx = 0 \quad (39)$$

and approximately, for the lower modes,

$$\int_0^L m y_i y_j dx = 0, \quad i \neq j \quad (40)*$$

Murray [71] by application of integration by parts to equations (38) and (39) discovered another interesting orthogonality relation, which has apparently not been noted previously in the literature,

$$\int_0^L y_i' dx \int_0^x m (\xi - \bar{x}) d\xi = 0 \quad (41)$$

Equation (41) is used by Murray and the authors to evaluate the constant of integration of  $\int \gamma' dx$ , and thus exactly determine the location of the true slope axis (This axis has previously been located arbitrarily at the mid-length point of the slope curve).

\* The reader is referred to Appendix D for the derivation of the exact inter-mode orthogonality conditions and for a discussion as to the applicability of the approximate condition, Equation 40.



Equation (37) denotes an eigenvalue problem, with  $\omega^2$  being the eigenvalue. Solutions will exist only for a discrete set of  $\omega^2$ . For each such value of  $\omega^2$ , a unique mode pattern will correspond. In many cases to this is the case when  $\omega^2 = 0$ , where two modes exist:  $\gamma = 1$  (rigid body translation), and  $\gamma = 2 - \frac{x}{L}$  (rigid body rotation). The two modes are orthogonal and have been orthogonalized for convenience.

Using orthogonality relations with respect to the normal modes patterns, the restriction for each

$$(38) \quad \int_0^L \omega^2 \gamma^2 dx = 0$$

$$(39) \quad \int_0^L \omega^2 \gamma dx = 0$$

and approximately, for the lowest modes,

$$(40) \quad \int_0^L \omega^2 \gamma^2 dx = 0, \quad i \neq j$$

Equation (41) by substitution is integrated by parts to equations (38) and (39) discovered another interesting orthogonality relation which has apparently not been noted previously in the literature,

$$(41) \quad \int_0^L \gamma_i' \gamma_j' dx = \left( \frac{1}{2} - \gamma_i \right) \delta_{ij} = 0$$

Equation (41) is used in many and the authors to evaluate the constant of integration of  $\gamma_i'$ , and this exactly determines the function in the case slope axis (this was not previously even pointed out in the half-length point of the slope axis).

\* The reader is referred to Appendix B for the derivation of the exact total mode orthogonality conditions and for a discussion of the approximate mode orthogonality conditions.



If we take  $y' = \eta' + C_3$  and substitute in equation (41), it is easily seen that  $C_3$  may be evaluated as

$$C_3 = \frac{\int_0^L \eta' \int_0^x m (\xi - \bar{x}) d\xi dx}{\int_0^L \int_0^x m (\xi - \bar{x}) d\xi dx}. \quad (42)$$

In a like manner, use is made of equation (38) to determine  $C_4$ , where  $y = \eta + C_4$ .

It must be emphasized that orthogonality among all the modes must be rigorously maintained. [2][71] A starting pattern for the iterative cycle is not strictly admissible unless it has been explicitly adjusted to conform with the orthogonality conditions described in equations (38), (39), and (40). The resultant pattern is not valid until the same adjustment procedure has been rigorously executed.

Orthogonality with respect to rigid body translation is insured by use of a zero shift of the y-axis:  $y_i = \bar{y}_i + \bar{A}$ . It is clear from equation (38) that:

$$\bar{A} \int_0^L m dx = - \int_0^L \bar{y}_i m dx. \quad (43)$$

Orthogonality with respect to rigid body rotation is insured by use of a rotation of the y-axis:  $y_i = \bar{y}_i + \bar{B} (x - \bar{x})$ . From equations (38) and (39) we may write

$$\int_0^L m y_i (x - \bar{x}) dx = 0, \quad (44)$$

and substitution of the above expression in equation (44) yields:

$$\bar{B} \int_0^L (x - \bar{x})^2 m dx = - \int_0^L m \bar{y}_i (x - \bar{x}) dx. \quad (45)$$

It is further required that both the starting and resultant deflection patterns be orthogonal with respect to each flexural mode prior to the one of immediate interest, we formulate the expression

If we take  $\gamma' = \gamma + C$  and substitute in equation (41), it is

easily seen that  $\gamma'$  may be evaluated as

$$\gamma' = \frac{\int_{-\infty}^{\infty} \frac{f(x)}{g(x)} dx}{\int_{-\infty}^{\infty} \frac{1}{g(x)} dx} \quad (42)$$

In a like manner, one is made of equation (41) to determine  $C$ , where

$$\gamma = \gamma' + C$$

It must be emphasized that orthogonality among all the modes must be rigorously maintained. [1] [1] A starting pattern for the iterative cycle is not strictly admissible unless it has been explicitly adjusted to conform with the orthogonality conditions demanded in equations (3). (3), and (4). The resultant pattern is not valid until the same adjustment procedure has been rigorously executed.

Orthogonality with respect to rigid body translation is insured by

$$\int_{-\infty}^{\infty} \gamma' dx = 0 \quad \gamma' = \gamma + \tilde{\gamma} \quad \tilde{\gamma} \text{ is clear from}$$

equation (3) that

$$\int_{-\infty}^{\infty} \tilde{\gamma} dx = - \int_{-\infty}^{\infty} \gamma dx \quad (43)$$

Orthogonality with respect to rigid body rotation is insured by

$$\int_{-\infty}^{\infty} \gamma' x dx = 0 \quad \gamma' = \gamma + \tilde{\gamma} \quad \tilde{\gamma} = \tilde{\gamma}(x)$$

From equations (3) and (3) we may write

$$\int_{-\infty}^{\infty} \gamma' x dx = \int_{-\infty}^{\infty} \gamma x dx + \int_{-\infty}^{\infty} \tilde{\gamma} x dx = 0 \quad (44)$$

and substitution of the above expression in equation (44) yields

$$\int_{-\infty}^{\infty} \gamma x dx = - \int_{-\infty}^{\infty} \tilde{\gamma} x dx \quad (45)$$

It is further required that both the starting and resultant deflection

patterns be orthogonal with respect to each other in the sense that

one of the modes must be, we formulate the expression

$$y_i = \bar{y}_i + \bar{A} + \bar{B} (x - \bar{x}) + C_1 y_1 + C_2 y_2 + \dots + C_{i-1} y_{i-1} \quad (46)$$

Constants  $C_1, C_2$  etc are readily determined by substituting  $y_1 = \bar{y}_1 + C_1 y_1$ , etc. into equation (40). For example, if we are interested in adjusting the second mode so that it is truly normal to the first mode we set

$$y_2 = \bar{y}_2 + C_1 y_1$$

whence from (40)

$$C_1 \int_0^L m y_1^2 dx = - \int_0^L m \bar{y}_2 y_1 dx \quad (47)$$

In general

$$C_i \int_0^L m y_j^2 dx = - \int_0^L m \bar{y}_i y_j dx \quad (48)$$

Not until the operations indicated by equation (46) are carried out should a cycle be begun nor a resultant deflection curve be accepted. Since the frequency depends upon the value of the resultant curve at the normalizing point,  $y_i^*$ , the frequency should be calculated only after orthogonalization is completed.

$$(45) \quad \bar{y}_i = \bar{y}_1 + \bar{y}_2 + \bar{y}_3 + \dots + \bar{y}_n + \bar{y}_{n+1} + \dots + \bar{y}_{n+i-1} + \bar{y}_{n+i}$$

Constants  $C_1, C_2$  are readily determined by substituting  $y = \bar{y}_1 + C_1 \bar{y}_2 + C_2 \bar{y}_3 + \dots$  into equation (45). For example, if we are interested in adjusting the second mode so that it is truly neutral to the first mode we set

$$y_2 = \bar{y}_2 + C_1 \bar{y}_1$$

where from (45)

$$(46) \quad C_1 \int_0^L \bar{y}_1 \bar{y}_2 dx = - \int_0^L \bar{y}_1 \bar{y}_2 dx$$

is general

$$(47) \quad C_i \int_0^L \bar{y}_i \bar{y}_j dx = - \int_0^L \bar{y}_i \bar{y}_j dx$$

Not only the equations indicated by equation (46) are derived but should a point be taken not a constant correction curve is needed. Since the frequency depends upon the value of the correction curve at the normalizing point,  $\bar{y}_i$ , the frequency should be calculated only after orthogonalization is completed.



Part IIDetailed Outline of Procedure :

This section supplements the general outline of the procedure of this thesis presented in Section II, and sets down the mechanics of the operations implementing the relationships derived in the theoretical section, Part I of this Appendix.

Some remarks helpful in interpreting the analytical expressions and the curves of Appendix B are in order. Roman numerals indicate figures in Appendix B.

- (1) All curves plotted from data given or calculated are plotted at intervals of 0.025  $L$  and faired through all spots. This is considered to give the least number of stations which will give a sufficiently accurate determination of true curve shape.
- (2) Since the length coördinate has been non-dimensionalized ( $\xi = x/L$ ), each integration performed with respect to it results in a factor of  $L$  in the denominator of the integral expression. These factors are removed at two points in the cycle: when determining curvature, and when normalizing deflection. However, their presence within the cycle results in appropriate modification of correction terms calculated to add to functions in the cycle.
- (3) Since the virtual mass distribution of the starting data was in pound units, the  $m$  of the theoretical relationships has been replaced by  $mg$ , the  $g$  (acceleration of gravity) has been carried throughout as a constant and is removed at the normalizing stage and in the frequency determination. The  $g$  constant necessarily appears also in the various auxiliary functions involving  $m$ .

## Part II

Detailed Outline of Procedure :

This section summarizes the general outline of the procedure of this thesis presented in section II, and sets down the mechanics of the equations implementing the relationships derived in the theoretical section, Part I of this Appendix.

Some remarks helpful in interpreting the analytical expressions and the curves of Appendix B are in order. Roman numerals indicate figures in Appendix B.

(1) All curves plotted from data given or calculated are plotted at intervals of 0.05  $\lambda$  and fitted through all points. This is considered to give the best manner of station which will give a sufficiently accurate determination of the curve shape.

(2) Since the length coordinate has been non-dimensionalized ( $x = x/\lambda$ ), each integration performed with respect to it results in a factor of  $\lambda$  in the denominator of the integral expression. These factors are removed at two points in the cycle: when determining curvature, and when normalizing deflection. However, their presence within the cycle results in appropriate modification of correction terms calculated to add to functions in the cycle.

(3) Since the virtual mass distribution of the testing data was in hand while the  $m$  of the theoretical relationships has been replaced by  $m/g$ , the  $g$  (acceleration of gravity) has been carried throughout as a constant and is removed at the normalizing stage and in the frequency determination. The  $g$  constant necessarily appears also in the various auxiliary functions

involving  $m$ .



(4) The horizontal backstepping of the various integral curves in the figures of Appendix B is inherent in the operation of the mechanical integrator used. (See Plate I) For convenience, the backstep has been set at integral numbers of inches, either two, three, or four inches, and is readily apparent from the y-axis labelled for each integral curve. Coördinates along the horizontal axis have not been renumbered for each axis shift, but should be transposed mentally in examining integral curves. Vertical shifts of integral curves are made where presentation is facilitated thereby.

(5) Also inherent in the integrator is a scale reduction of the integral curve with respect to its argument. Scale factors used in this investigation ranged from three to 7.5; while not indicated directly on the curves, they of course determine the scales marked on the integral axes. The continuous maintenance of correct scales is vital to accurate determination of frequency from the deflection normalizing factor.

(6) Figures illustrating the method have been so arranged that the four principal integrations comprising a single cycle appear in one figure (e. g. Figure VI) while the auxiliary curves pertaining to the same cycle appear on one or two separate sheets following (e. g. Figure VII). At the end of each mode is a figure ( XII, XX, XXXII) illustrating the convergence of the normalized deflection patterns from the orthogonalized uniform beam pattern ultimately deemed acceptable as the normal mode pattern.

#### Sequence of Operations:

When the tables and smooth curves (I) of structural information have been prepared, it is necessary to obtain the coördinate of the longitudinal centroid of the appropriate virtual mass. This may be done by

(4) The horizontal differentiation of the various integral curves in the figures of Appendix II is indicated in the caption of the mechanical integrals used. (See Plate I) For convenience, the caption has been set at integral numbers of inches, since the paper, as you know, and is readily apparent from the figures labeled for each integral curve. Conclusions along the horizontal axis have not been recommended for each unit, but should be expressed mainly in terms of integral curves. Vertical shifts in integral curves are made where continuation is facilitated thereby.

(5) Also indicated in the integrals is a scale criterion of the integral curve which respects its signum. Scale factors used in this investigation ranged from one to 7.5, while not indicated directly on the curves. They of course determine the scales marked on the integral axes. The continuous maintenance of correct scales is vital in accurate determination of frequency from the deflection measuring device.

(6) Figures illustrating the method have been so arranged that the last practical integrations concerning a single cycle appear in the figure (e.g. Figure VII) while the auxiliary curves remaining in the same figure appear as two separate sheets (e.g. Figure VIII). At the end of each mode is a figure (XII, XIII, XIV) illustrating the convergence of the normalized deflection pattern from the unnormalized pattern. These figures consistently demand acceptance as the normal mode pattern.

Dependence of Operation

When the tables and graphs (I) of structural information have been prepared, it is necessary to give the conditions of the total condition of the appropriate virtual mass. This may be done by



any standard technique, but figure II illustrates a simple and unique method. The tangent to the end of the second integral of the mass distribution is extended to the base line, and the resulting intersection, relative to the origin of the second integral curve, is the abscissa of the longitudinal centroid of the mass. The proof of this is given in [7].

Sufficient information is now on hand to prepare the tables of working functions, including

$$mg, m g L/L^2 A, 10^6 L^2/EI, KAG/10^6, (\xi - \bar{x})/L, \\ m g (\xi - \bar{x})/L, \text{ and } g \int_0^x m (\xi - \bar{x}) d\xi / L^2$$

As they become known, there will be added to the working functions table the following tabulated functions:

$$y_1, y_1^m, y_2, y_2^m, \text{ etc.}$$

This table is prepared for intervals of 0.025 L.

Certain definite integrals and other constants useful throughout the application of the method are also calculated and listed at this point:

$$\Delta/L = \int_0^L m g dx/L \quad (49)$$

$$N = \int_0^L (x - \bar{x})^2 m g dx/L^3 \quad (50)$$

$$P = \int_0^L \int_0^x (\xi - \bar{x}) m g d\xi dx/L^4 \quad (51)$$

Subsequently, when data is available, the following are added:

$$W_1 = \int_0^L y_1^2 m g dx/L \quad (52)$$

$$W_2 = \int_0^L y_2^2 m g dx/L, \text{ etc.} \quad (53)$$

It is now possible to commence the process. While any approximation to the mode shape can be used as the starting curve, it is found that the most accurate approximation to the final mode shape available will lead to accelerated convergence, hence fewer cycles of



iteration for the desired accuracy.

The proposal here is to take a known pattern, as from a similar ship or a uniform beam, and force it to comply with conditions of orthogonality applicable to the ultimate normal pattern. Thus in the investigation conducted for this thesis, the normalized ordinates of the patterns found by Lord Rayleigh are taken arbitrarily as a starting point.

The following description traces the development of a third, or four-noded mode, to demonstrate the typical case of a higher mode.

From the working function table and the values of  $Y_3$ , tabulations are obtained and plotted of  $mgY_3$ ,  $mg(x - \bar{x}) Y_3/L$ ,  $mg y_1 Y_3$ , and  $mg y_2 Y_3$ . The four resulting curves are integrated (XXII), and the values obtained are divided by the operational constants indicated to obtain

$$\bar{A} = - \frac{\int_0^L mg Y_3 dx/L}{\Delta/L} \quad (54)$$

$$\bar{B} = - \frac{\int_0^L mg (x - \bar{x}) Y_3 / L}{N} \quad (55)$$

$$C_i = \frac{-\int_0^L mg y_1 Y_3 dx/L}{W_1} \quad (56)$$

$$C_{ii} = \frac{-\int_0^L mg y_2 Y_3 dx/L}{W_2} \quad (57)$$

Now the values of  $(x - \bar{x})/L$  are multiplied by  $\bar{B}$ ; the values of  $y_1$  are multiplied by  $C_i$ ; and the values of  $y_2$  are multiplied by  $C_{ii}$ . Then we obtain

$$\bar{y} = \bar{A} + \bar{B} (x - \bar{x})/L + C_i y_1 + C_{ii} y_2 \quad (58)$$

All values of  $\bar{y}$  are divided by  $y^*$ , its value at its normalizing point (the center-or centermost-after antinode as obtained by a plot of that region)



iteration for the desired accuracy.

The proposed test is to take a known function, or from a similar ship or a uniform mass, and take it to comply with conditions of other quantities applicable to the unknown function. Then in the following iteration for this mass, the normalized ordinates of the function from corrected for this mass, are taken as  $Y_1$  and  $Y_2$  as a starting point.

The following mass, varied over the development of a load, or

load-carrying mass, or momentum, the typical case of a single mode.

From the known function  $Y_1$  and the value of  $Y_2$  the values

are obtained and plotted at  $Y_1$ ,  $Y_2$ ,  $Y_3$ ,  $Y_4$ ,  $Y_5$ ,  $Y_6$ ,  $Y_7$ , and  $Y_8$ . The load-carrying curves are integrated (AREA) and the values obtained are divided by the operational constants indicated in Table

(21)

$$Y_1 = \frac{m g \cdot Y_1 \cdot \Delta}{\Delta}$$

(22)

$$Y_2 = \frac{m g \cdot Y_2 \cdot \Delta}{\Delta}$$

(23)

$$Y_3 = \frac{m g \cdot Y_3 \cdot \Delta}{\Delta}$$

(24)

$$Y_4 = \frac{m g \cdot Y_4 \cdot \Delta}{\Delta}$$

(25)

Now the value of  $(Y_1 - Y_2)$  is multiplied by  $\Delta$ , the value

of  $Y_1$  are multiplied by  $\Delta$ , and the value of  $Y_2$  are multiplied by  $\Delta$ . Then

we obtain

(26)

$$Y = Y_1 + Y_2 + Y_3 + Y_4 + Y_5 + Y_6 + Y_7 + Y_8$$

All values of  $Y$  are divided by  $Y_1$ , the value in the normalizing point (the center of gravity-mass) and the values are divided by a mass of (see Table)

to obtain a tabulation of  $\gamma$ , the orthogonalized, normalized starting curve for the first cycle of iteration. (But see discussion of orthogonality in Appendix D).

The cycle itself (XXIII) commences with the curve of inertia loading  $= -\gamma m g$ , which is calculated, plotted, and integrated, to give a function of shear,

$$g_{Q/L} = -\gamma m g \quad (59)$$

where the constant of integration is zero, since the integral curve starts at the origin, and the boundary condition of zero end shear dictates that the true shear curve should also pass through the origin. However, the right end of the integral curve may differ from zero by a small amount (usually less than 5% of the maximum ordinate of the curve).

When this occurs, a linear correction is applied to the curve to again conform to the above boundary condition. This is done by drawing a new axis joining the ends of the shear curve, measuring its ordinates from this curve but normal to the old axis, then replotting on the old axis. This correction is illustrated in Figure VI, where the reader should compare the curves marked  $g_{Q/L}$  and  $g_{Q/L}$  (corrected), but has been omitted from the presentation elsewhere to avoid congesting the plots unnecessarily. In the first cycle, the corrected curve gives  $g_{M'/L} = g_{Q/L}$ .

(At this point in all but the first cycle, each value of shear receives an additive correction to account for rotatory inertia, the correction being a function of the bending slope  $\gamma$  obtained from the previous cycle in a manner to be described. This slope, in the form  $L\gamma$ , is multiplied by the working function  $m g I/L^2 A$  and the product added to the shear term to give a curve of

$$g_{M'/L} = g_{Q/L} + \gamma g m I / L A \quad (60)$$



to obtain a tabulation of  $\gamma$ , the orthogonally, normalized starting curve for the first cycle of iteration. This one discussion of orthogonality is Appendix D.

The cycle itself (XXIII) commences with the curve of loads  $\gamma = \gamma_{\text{orig}}$ , which is calculated, plotted, and integrated, to give a function of shear,

$$(29) \quad \gamma_{\text{int}} = \gamma_{\text{orig}}$$

where the constant of integration is zero, since the integral curve starts at the origin, and the boundary condition at zero and shear illustrates that the new shear curve should also pass through the origin. However,

the right end of the integral curve may differ from zero by a small amount (usually less than 2% of the maximum ordinate of the curve).

When this occurs, a linear correction is applied to the curve to again conform to the same boundary condition. This is done by drawing a new axis joining the ends of the shear curve, measuring its ordinate from this curve out normal to the old axis, then rotating on the old axis.

This correction is illustrated in Figure VI, where the reader should compare the curves marked  $\gamma_{\text{int}}$  and  $\gamma_{\text{int}}$  (corrected), but has been omitted from the presentation elsewhere to avoid congesting the plots unnecessarily. In the first cycle, the corrected curve gives  $\gamma_{\text{int}} = \gamma_{\text{int}}$ .

(At this point in  $\gamma$  but the first cycle, each value of shear receives

an additive correction to account for rotary inertia. The correction being a function of the bending slope  $\gamma$  obtained from the previous cycle in a manner to be described. This slope, in the form  $\gamma$ , is multiplied by the working function  $w \gamma^2$  and the product added to the shear before giving a curve of

$$(30) \quad \gamma_{\text{int}} = \gamma_{\text{int}} + w \gamma^2$$



This is the only procedural respect in which subsequent cycles differ from the first).

The curve of  $g M'/L$  is integrated, and the boundary condition of zero end moment is applied in exactly the manner described above for the shear correction. In both cases the correction can be considered to filter out accumulated errors resulting from small inaccuracies of integrator operation and calculation. The values of  $g M, L^2$ , a direct function of moment in the hull girder, are tabulated from the corrected curve, and each is multiplied by the corresponding value of  $10^6 L^2/EI$ , to give a function of curvature,  $g\gamma' \times 10^6$ , which is plotted.

Curvature is integrated to give bending slope to within an additive constant. To this component is added the shear component of slope,

$$g\beta \times 10^6/L = (g Q/L) \div (KAG/10^6) \quad (61)$$

based on the shear values obtained after correction of the first integral in the cycle.

The addition gives a function of total slope, still without the constant of integration:

$$g\eta' \times 10^6/L = g\bar{\gamma} \times 10^6/L + g\beta \times 10^6/L \quad (62)$$

To determine the constant, or axis shift, use is made of Murray's formulation (see Part I of this appendix) on an auxiliary plot (XXIV).

The values of  $g\eta' \times 10^6/L$  from the above addition are multiplied by the working function

$$\int_0^x m g (\xi - \bar{x}) d\xi / L^2$$

and the product plotted and integrated on the auxiliary plot. The value of the definite integral obtained,

$$10^6 g C_3/L = - \frac{\int_0^L g\eta' \times 10^6/L \int_0^x m g (\xi - \bar{x}) d\xi dx / L^3}{P} \quad (63)$$

This is the only procedure in which unambiguous cycles differ from the first.

The curve of  $g$  in  $y$  is integrated, and the boundary conditions of zero and moment is applied in exactly the manner described above for the shell correction. In both cases the correction can be considered to filter out accumulated errors resulting from small inaccuracies of integrals and operations and calculation. The values of  $g$  at  $y = 0$  and  $y = 1$  are then calculated from the corrected curve, and when it is multiplied by the corresponding value of  $10^{-5}$   $h$ , it gives a function of curvature,  $g \times 10^{-5}$ , which is plotted.

Curvature is integrated to give bending slope in which an additive constant. The this constant is added the shear component of slope,

$$g \times 10^{-5} \text{ in } y = (g \times 10^{-5}) \div (2.5 \times 10^{-5}) \quad (41)$$

based on the value obtained after integration of the first integral in the cycle.

The solution gives a function of total slope, with which the

constant of integration:

$$g \times 10^{-5} \text{ in } y = g \times 10^{-5} + g \times 10^{-5} \quad (42)$$

To determine the constant, we note that, as in case of Murphy's

formulation (see Table 1 of this report) or in equation (23IV), the value of  $g \times 10^{-5}$  is the same as the value obtained by the

working function

$$\int_0^1 g \times 10^{-5} dy = \int_0^1 g \times 10^{-5} dy$$

and the product slope and integration on the boundary plane. The value

of the definite integral obtained

$$g \times 10^{-5} \text{ in } y = \frac{\int_0^1 g \times 10^{-5} dy}{1} \quad (43)$$

is added to the uncorrected total slope to give the function of true slope:

$$g\gamma' \times 10^6/L = g\eta' \times 10^6/L + 10^6 g C_3/L \quad (64)$$

This is plotted on the cycle plot and integrated, yielding, to within an additive constant of integration, a function of deflection,

$g\eta \times 10^6/L^2$ . To obtain the constant, this is multiplied by  $mg$  and the product plotted on the auxiliary plot (XXIV) and integrated. The value of the definite integral is divided by a function of displacement to give the constant

$$10^6 g C_4/L^2 = - \frac{\int_0^x (g\eta \times 10^6/L^2) mg \, dx/L}{\Delta/L} \quad (65)$$

This is added to the values of  $g\eta \times 10^6/L^2$  to give the deflection pattern

$$g\gamma \times 10^6/L^2 = g\eta \times 10^6/L^2 + gC_4 \times 10^6/L^2 \quad (66)$$

This pattern is orthogonalized in the same manner as was described for the Rayleigh pattern, to give  $g\bar{\gamma} \times 10^6/L^2$ . The value at the normalizing point,  $g\gamma^* \times 10^6/L^2$ , is recorded, and divided into all ordinate values, to yield the normalized deflection pattern,  $\gamma_{3-1}$ , of the first cycle. It is this pattern which is used to compute the inertia loading for the next cycle of the iteration.

The value of  $g\gamma^* \times 10^6/L^2$  is multiplied by  $L^2$ , and the product resulting is divided into the values of  $g\gamma' \times 10^6$  from the last cycle of the iteration for each mode to yield the normalized curvature for the mode, which now will have the correct relationship to the normalized deflection pattern.

The value of  $\omega^2$  corresponding to the cycle just completed is

$$\omega^2 = \frac{1}{g\gamma^* \times 10/L^2} \quad (67)$$





The value of  $\omega^2$  computed from the last cycle performed is accepted as the natural frequency corresponding to the mode being considered.

In preparation for the rotatory inertia correction, the shear curvature is subtracted from the total curvature to give the corrected bending curvature

$$g\gamma \times 10^6/L = g\gamma' \times 10^6/L - g\beta \times 10^6/L \quad (68)$$

which is normalized by dividing by  $g\gamma' \times 10^6/L^2$  to yield the required value of  $\gamma L$ .





## APPENDIX B - SUMMARY OF DATA AND CALCULATIONS

Figures I through XXXIV in this section depict the application of the method developed in this thesis to the model for which characteristics are tabulated in Appendix E. The calculation of argument curves plotted from data is illustrated in the sample calculations of Appendix C.

Table III summarizes the effect of each iterative cycle, and its associated orthogonalization check, on the natural frequency of the mode considered. The corresponding effect on deflection patterns is presented in Figures XII, XX, and XXXII, for the first, second, and third modes respectively.

## APPENDIX E - SUMMARY OF DATA AND CALCULATIONS

Figure I through XXIV in this section depict the application of the method developed in this thesis to the model for which observations are tabulated in Appendix A. The calculation of argument curves plotted from data is illustrated in the sample calculations of Appendix C.

Table III summarizes the effect of each iterative cycle, and the associated organizational check, on the natural frequency of the mode considered. The corresponding effect on deflection patterns is presented in Figures XX, XI, and XXII for the first, second, and third modes respectively.

TABLE III  
 BEHAVIOR OF FREQUENCY FROM  
 CYCLE TO CYCLE

<u>MODE</u>	<u>CYCLE</u>	<u>FREQUENCY, cps.</u>
1	1	409.8*
	2	423.5
	3	417
2	1	865*
	2	840
	3	838.8
3	1	1507*
	2	1377
	3	1391

\* The first cycle of each mode necessarily does not include a consideration of rotatory inertia.



## TABLE III

REMARKS ON REMOVAL FROM

STUDY TO DATE

STUDY NO.	REMARKS	DATE
1001	1	1
1002	1	1
1003	1	1
1004	1	1
1005	1	1
1006	1	1
1007	1	1
1008	1	1
1009	1	1
1010	1	1

\* The first type of work was necessarily done and  
 therefore a consideration of various factors.

FIGURE I  
STRUCTURAL CHARACTERISTICS  
OF A 3/8 SCALE  
MODEL SUBMARINE

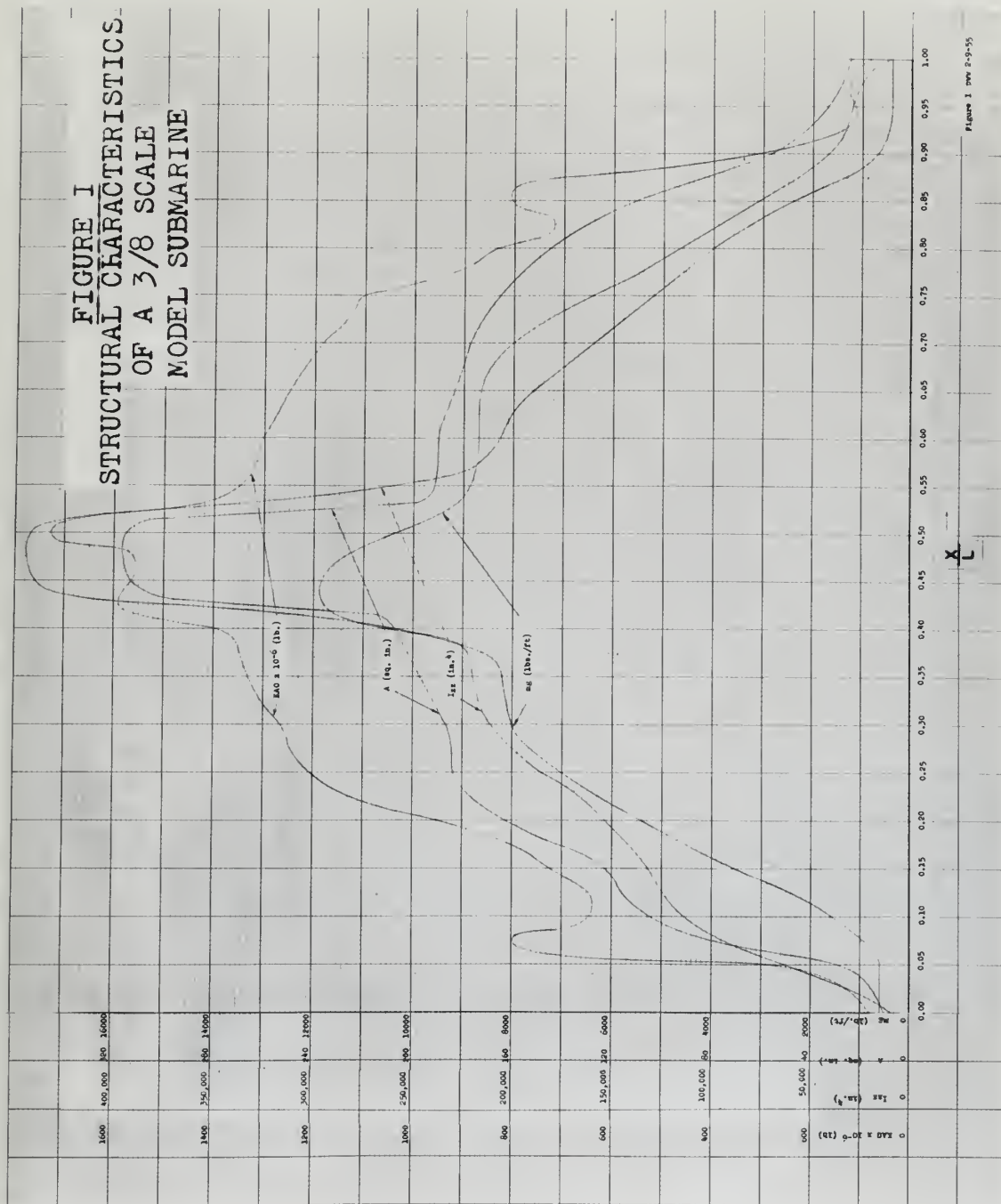


Figure 1 from 2-9-55





**FIGURE II**  
**DETERMINATION OF LONGITUDINAL CENTROID**  
**OF VIRTUAL MASS FOR TRANSVERSE VIBRATION**

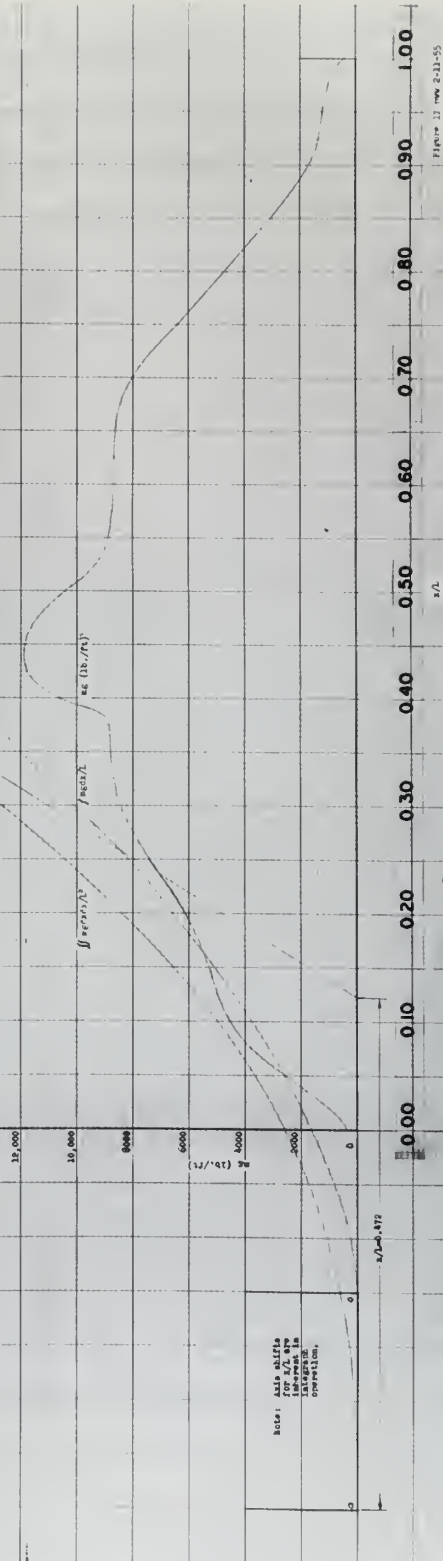










FIGURE IV  
RAYLEIGH MODE PATTERNS

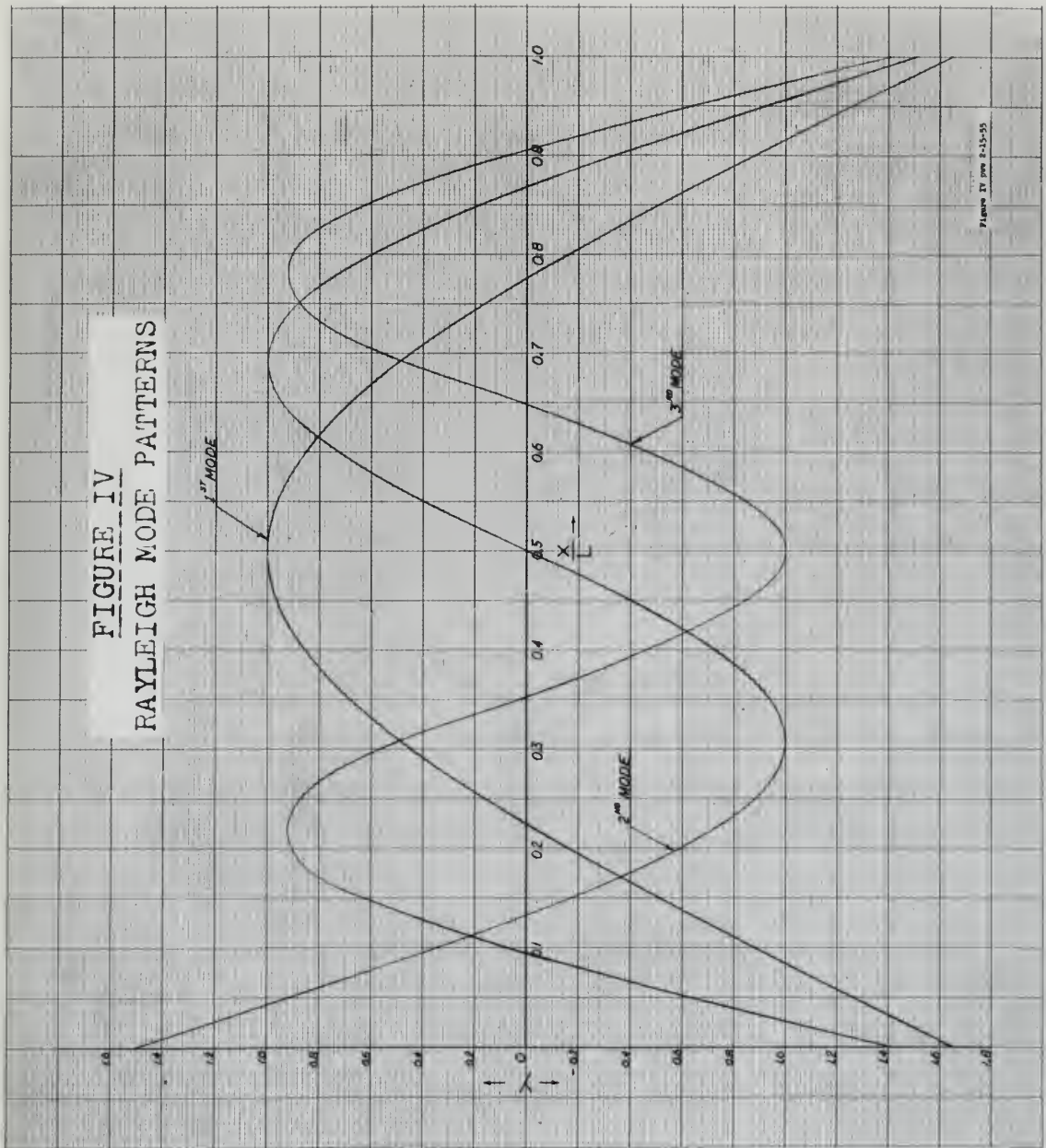














FIGURE VII  
ADJUSTMENT CONSTANTS  
FIRST CYCLE, FIRST MODE

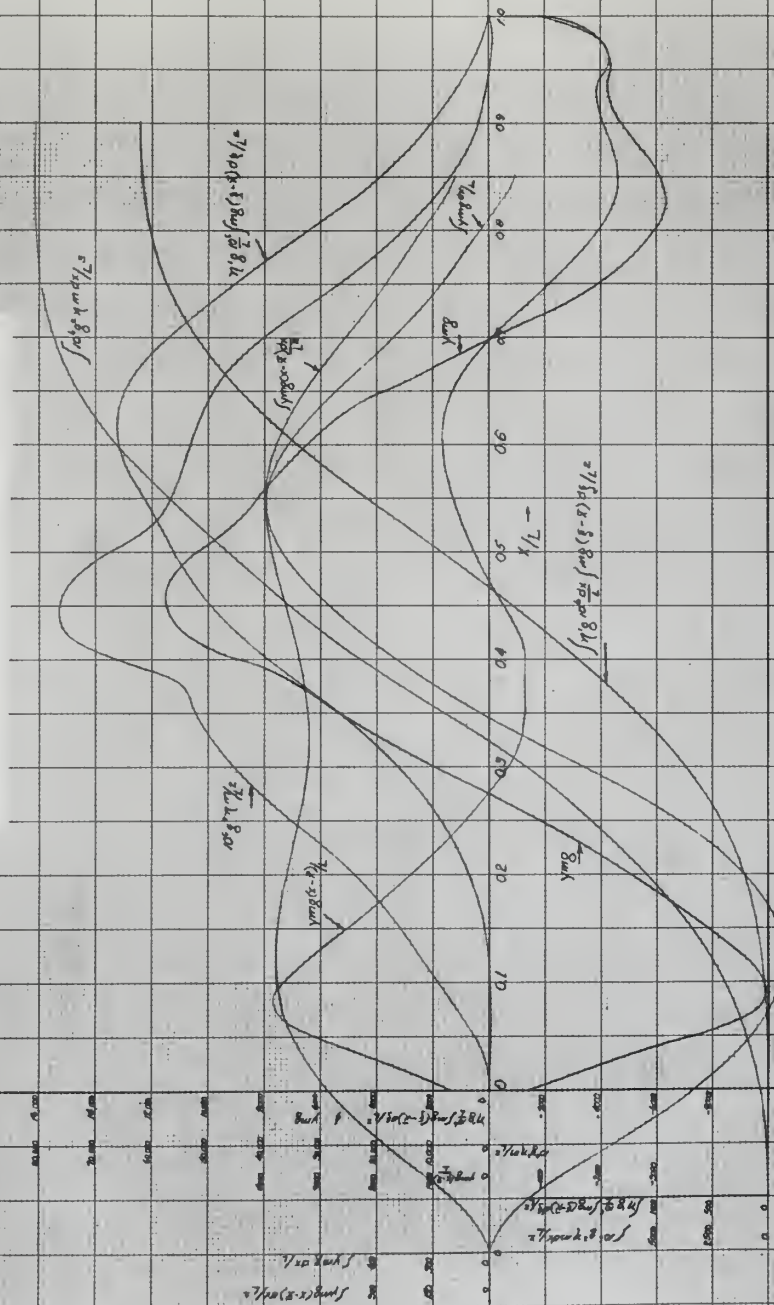






FIGURE VIII  
SECOND CYCLE OF FIRST MODE

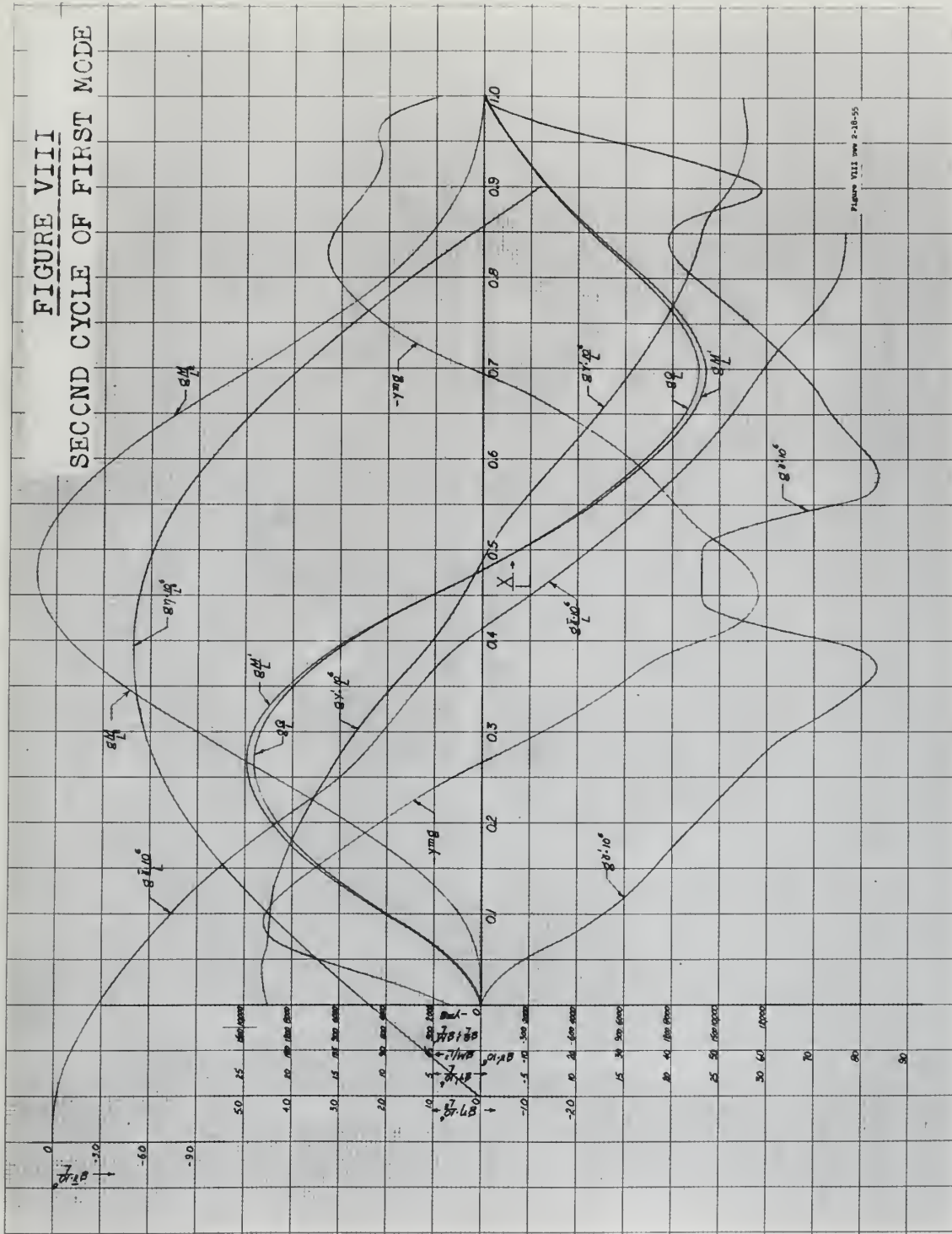




















FIGURE XII  
RESULT OF ITERATION  
FIRST MODE

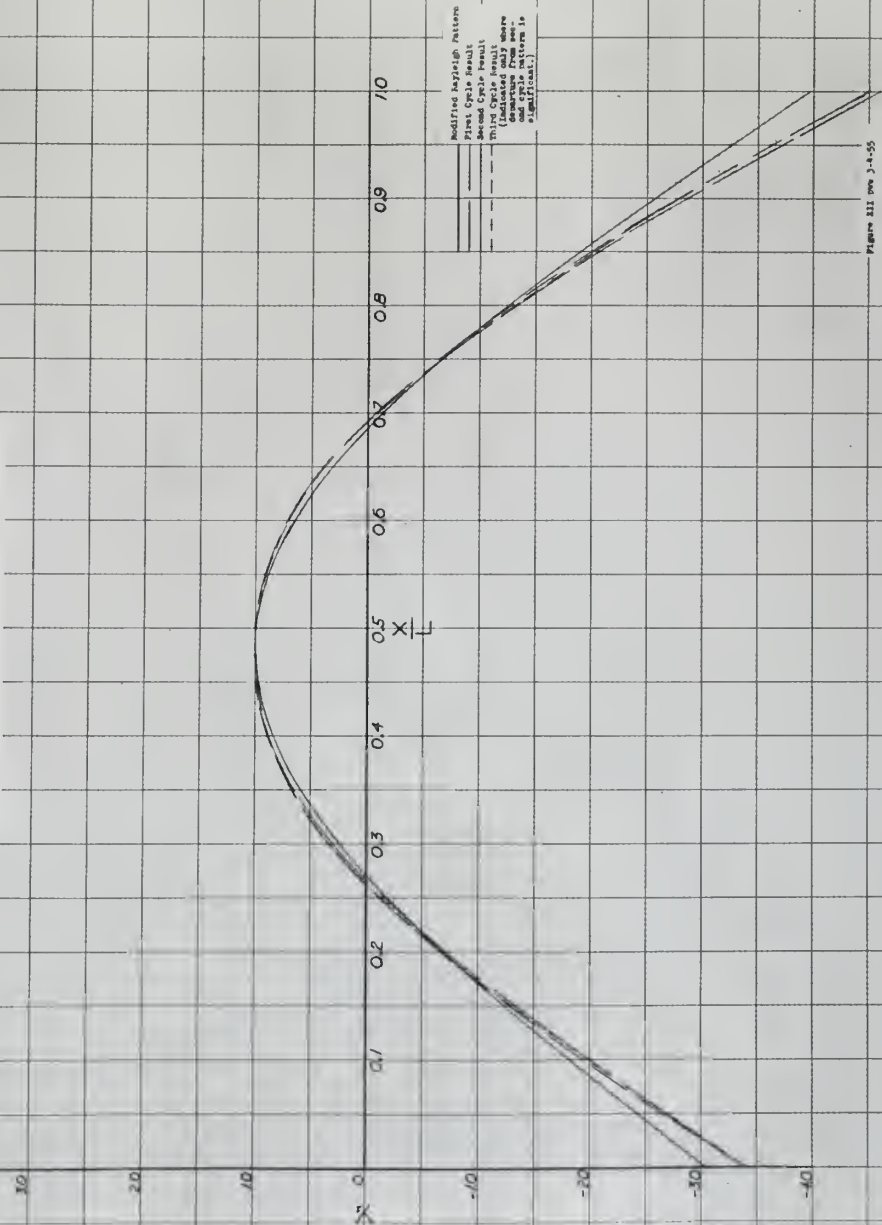
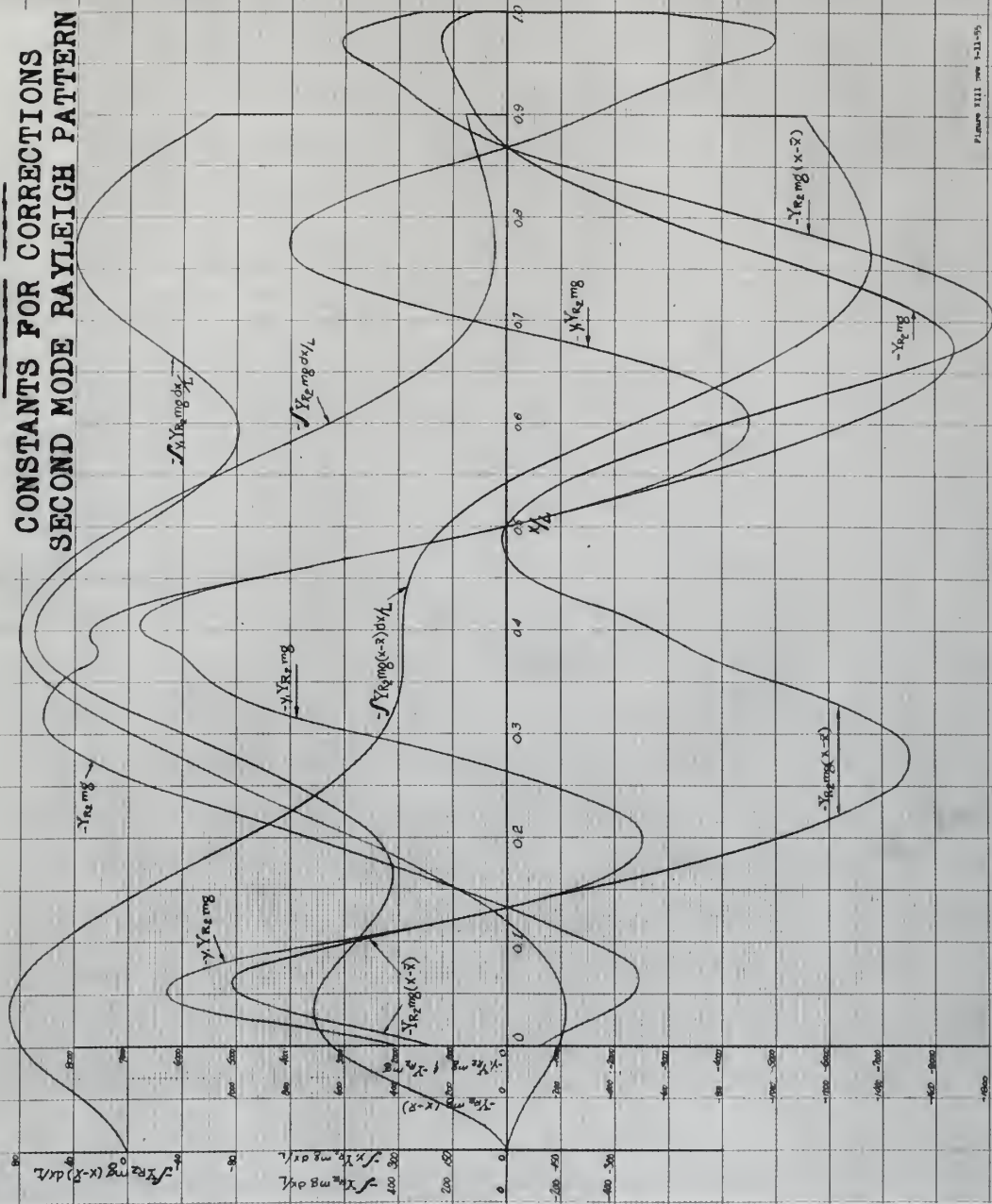


FIGURE XII OVER 3-4-55



**FIGURE XIII**  
**CONSTANTS FOR CORRECTIONS**  
**SECOND MODE RAYLEIGH PATTERN**







**FIGURE XIV**  
**FIRST CYCLE**  
**OF SECOND MODE**

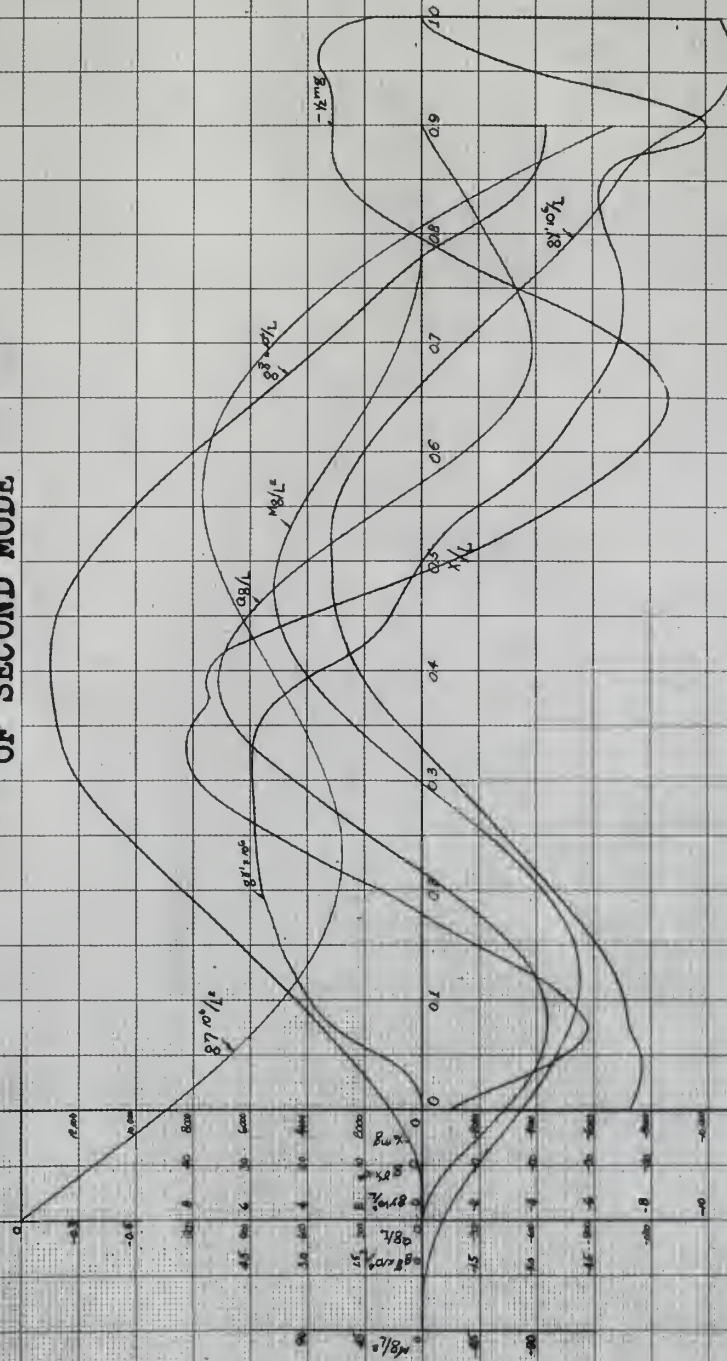


Figure XIV new 3-11-55

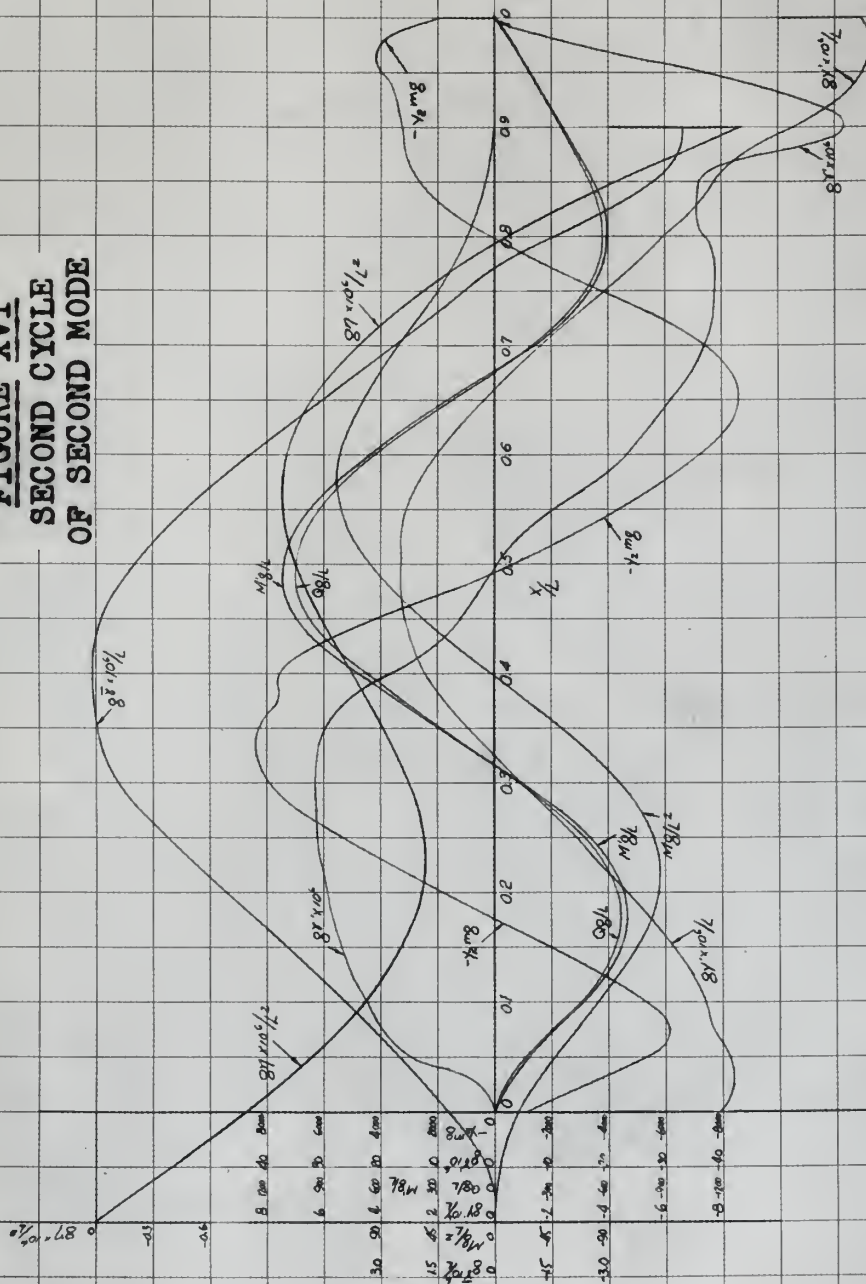








**FIGURE XVI**  
**SECOND CYCLE**  
**OF SECOND MODE**





**FIGURE XVII**  
**ADJUSTMENT CONSTANTS**  
**SECOND CYCLE, SECOND MODE**

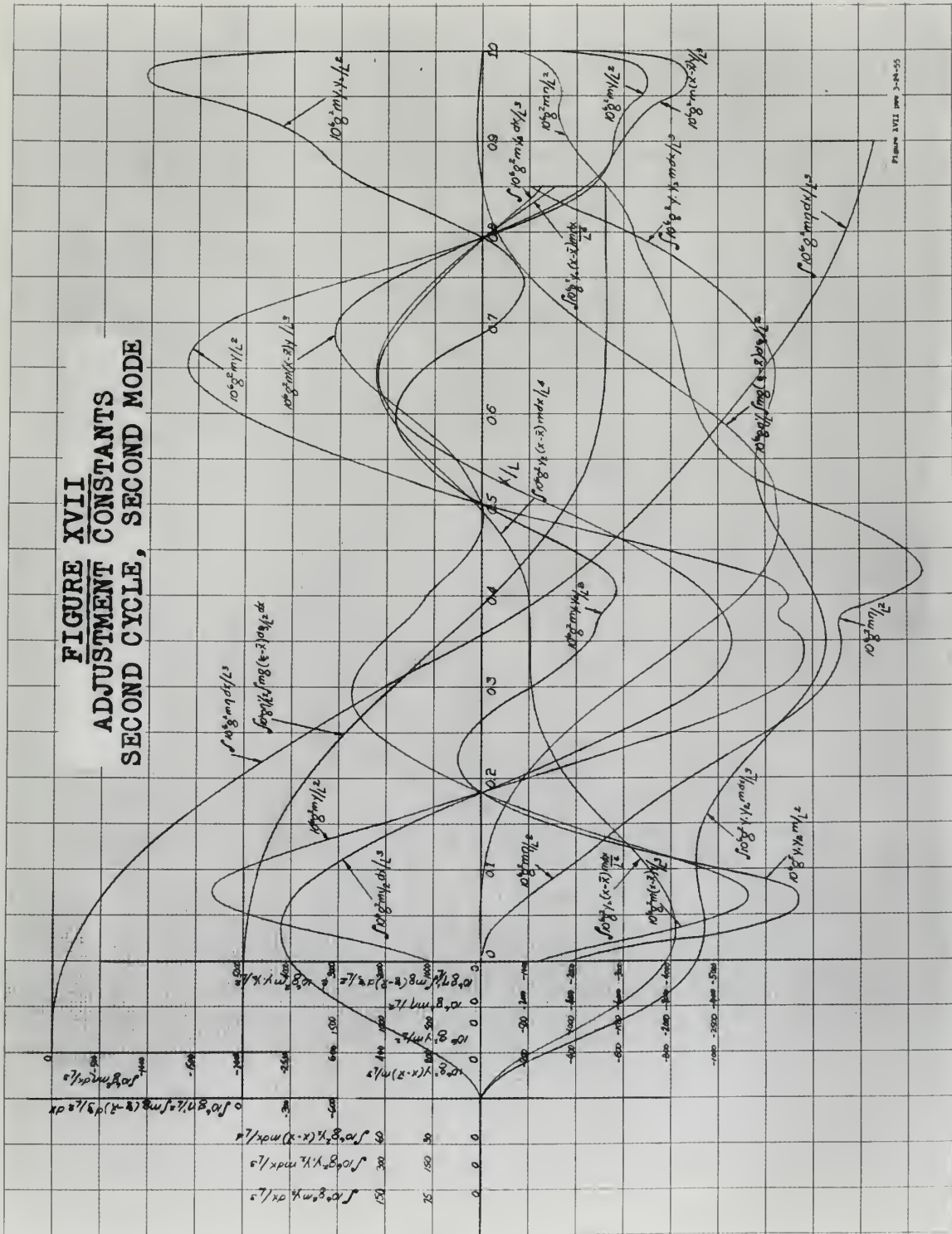


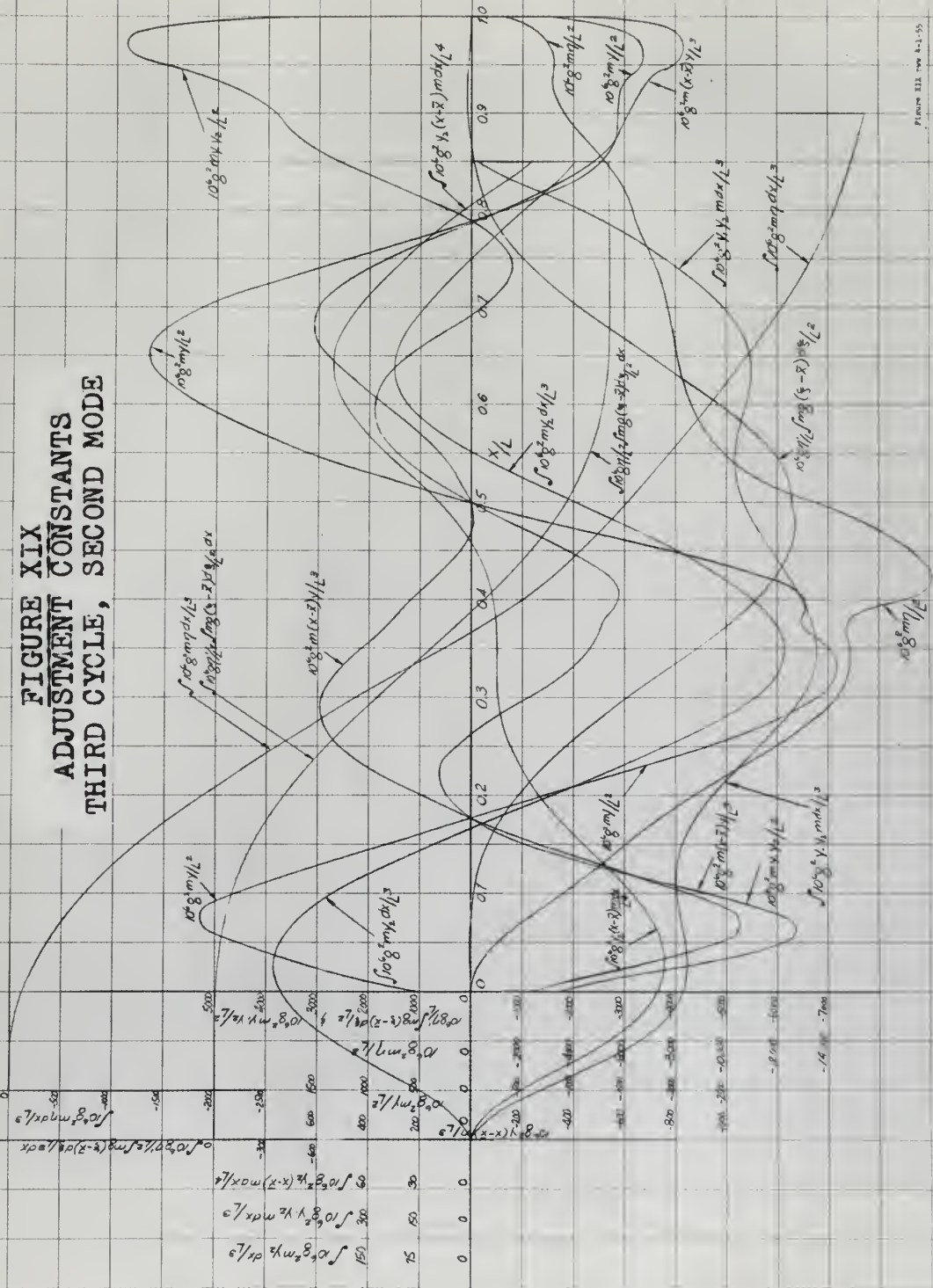








FIGURE XIX  
ADJUSTMENT CONSTANTS  
THIRD CYCLE, SECOND MODE







**FIGURE XX**  
**RESULTS OF ITERATION**  
**SECOND MODE**

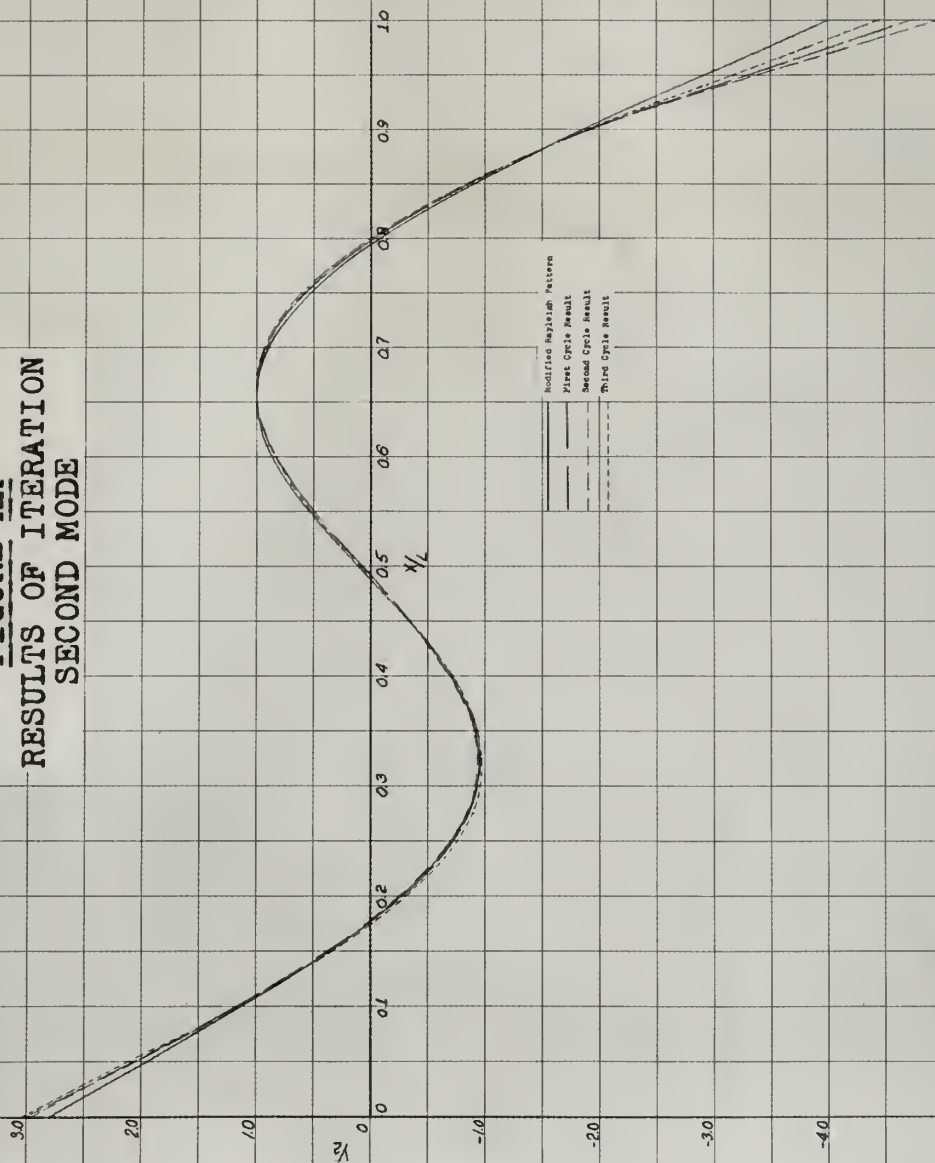


Figure XX 1990 9-2-55



**FIGURE XXI**  
**CONSTANTS FOR**  
**MODE CORRECTIONS**

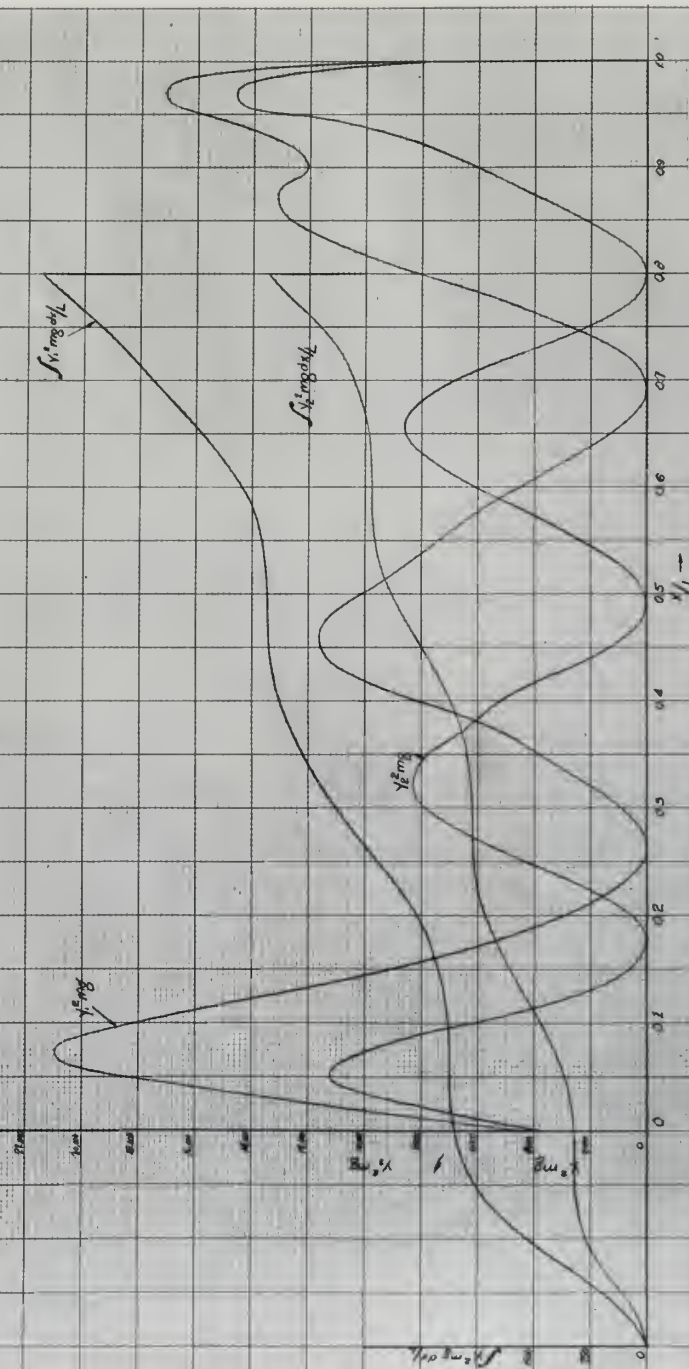


Figure XXI (cont. from 4-2-55)





**FIGURE XXII**  
**CONSTANTS FOR CORRECTIONS**  
**THIRD MODE RAYLEIGH PATTERN**

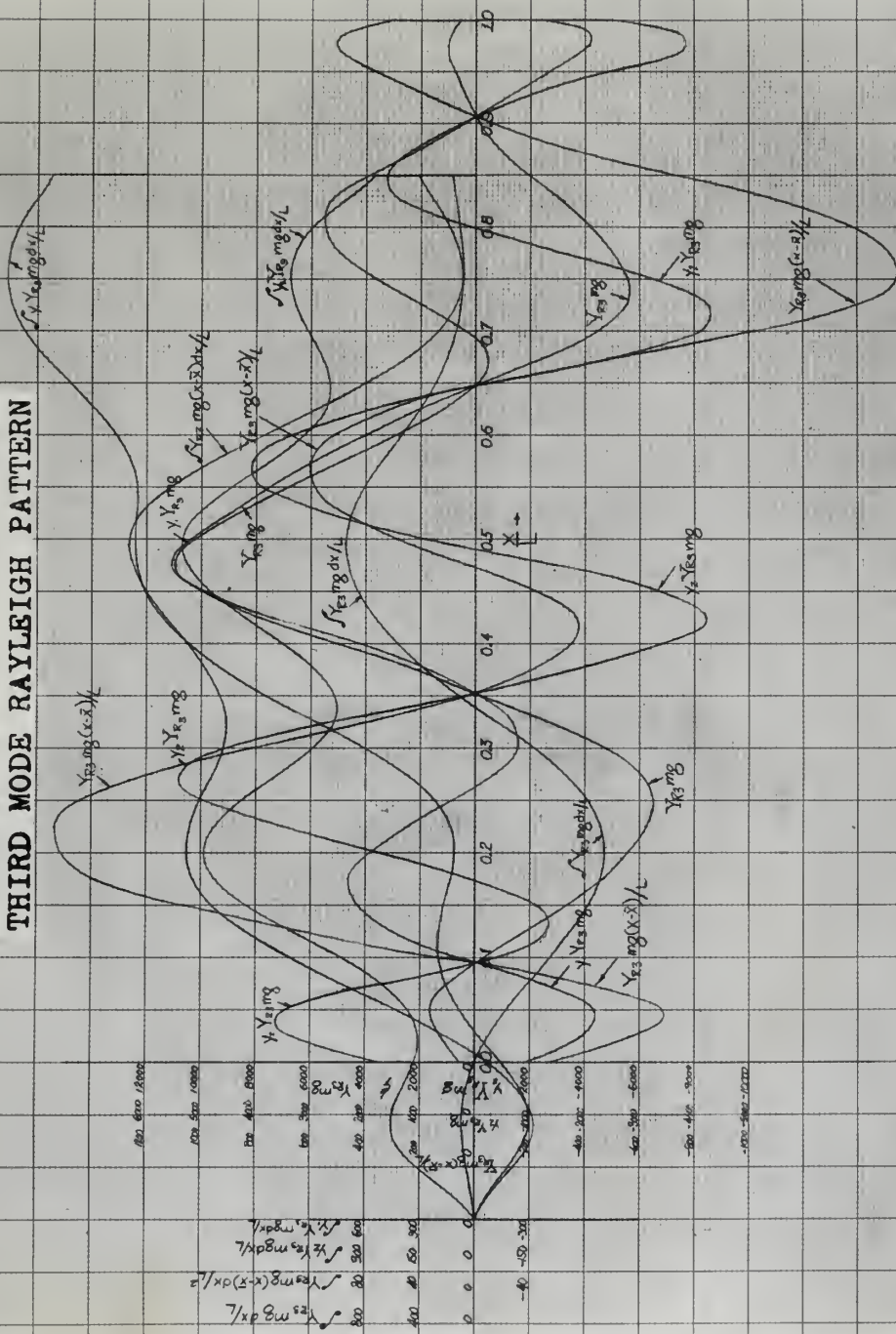










FIGURE XXIV  
CONSTANTS OF INTEGRATION  
FIRST CYCLE, THIRD MODE

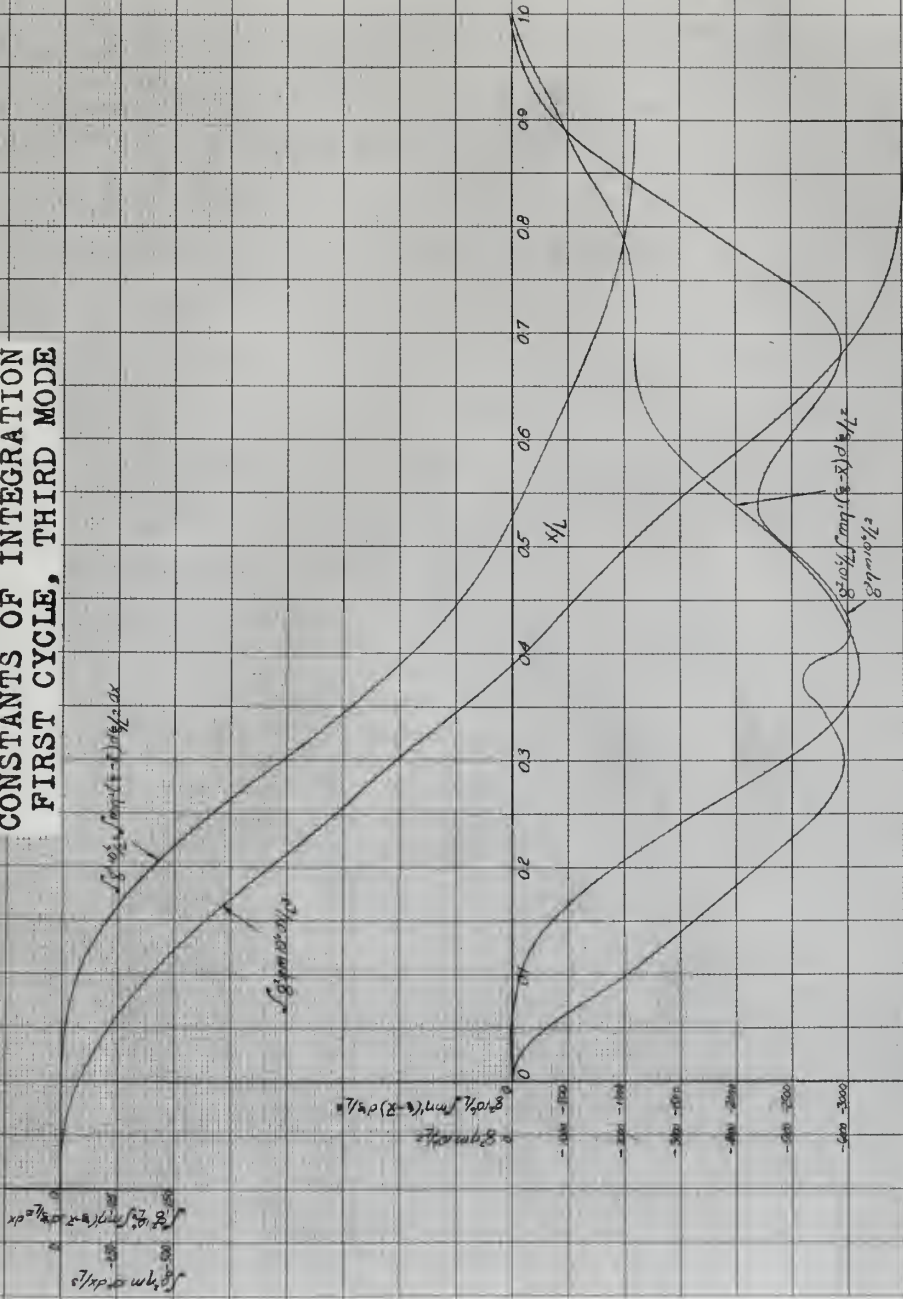


Figure IIIA per 6-7-85















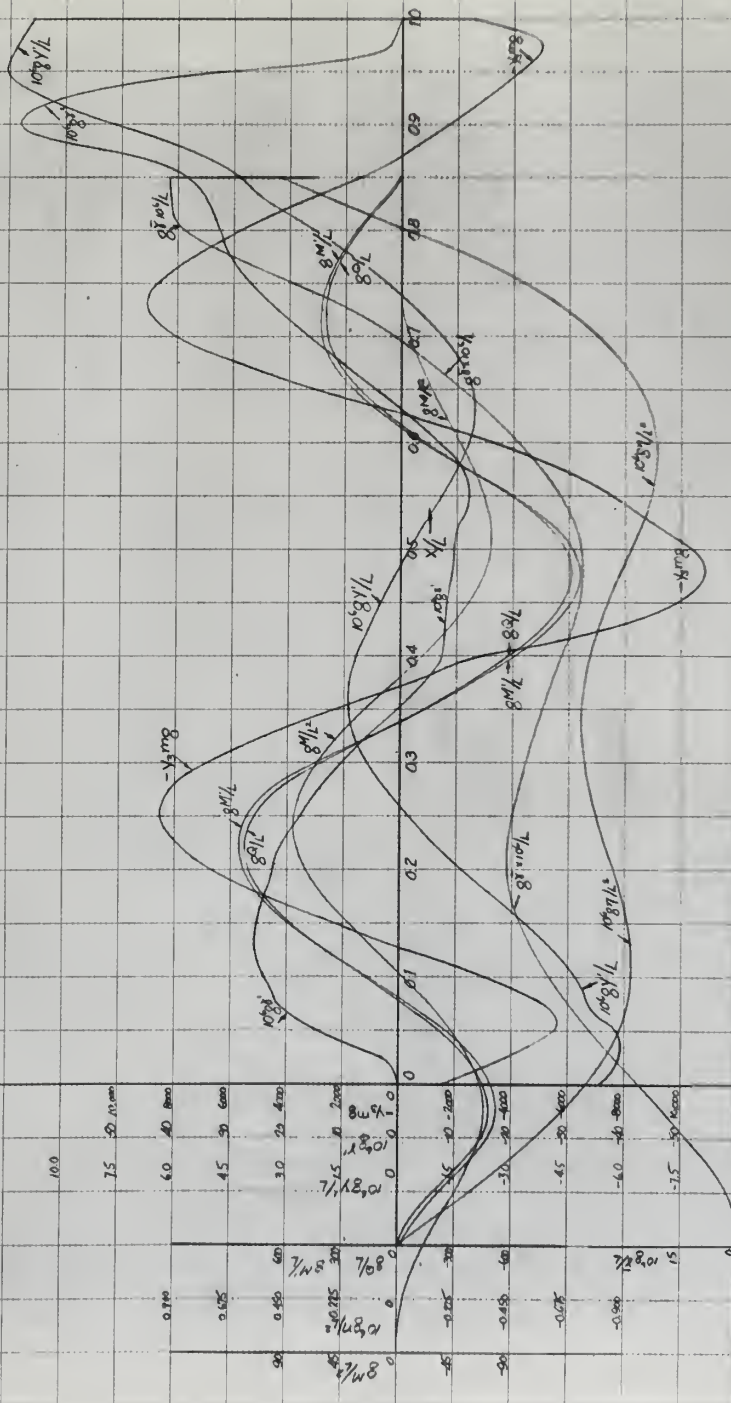








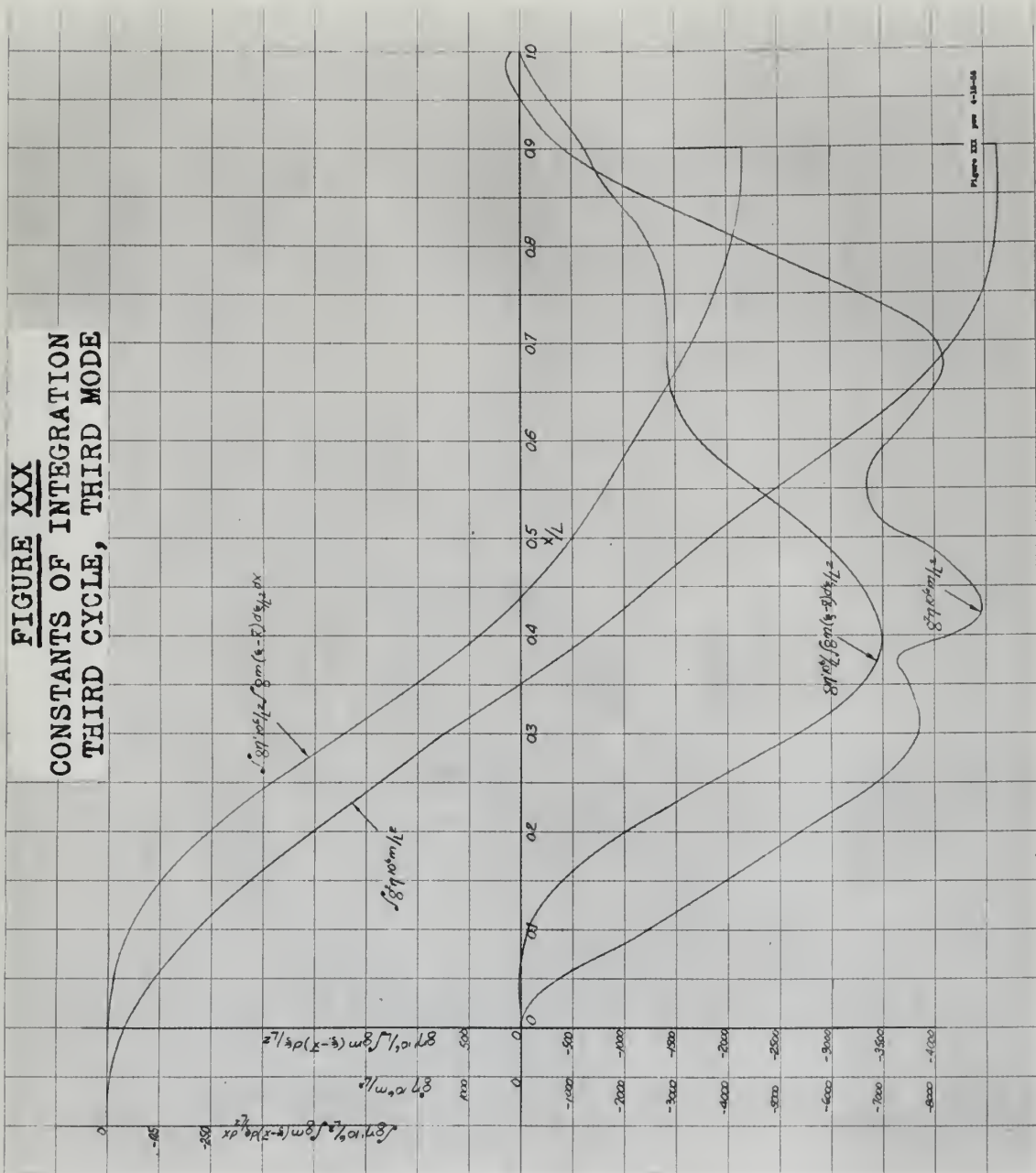
**FIGURE XXIX**  
**THIRD CYCLE**  
**OF THIRD MODE**







**FIGURE XXX**  
**CONSTANTS OF INTEGRATION**  
**THIRD CYCLE, THIRD MODE**



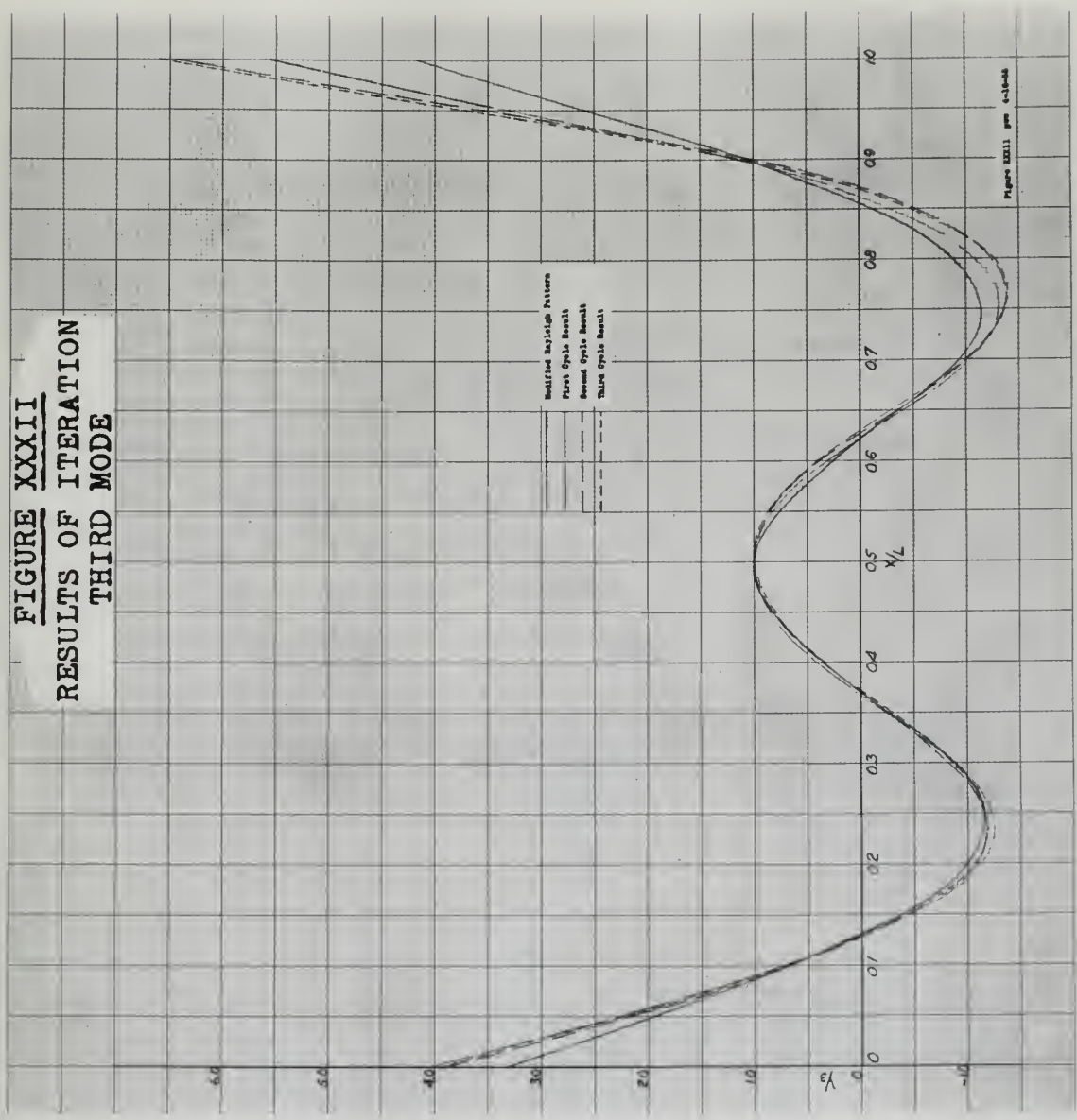






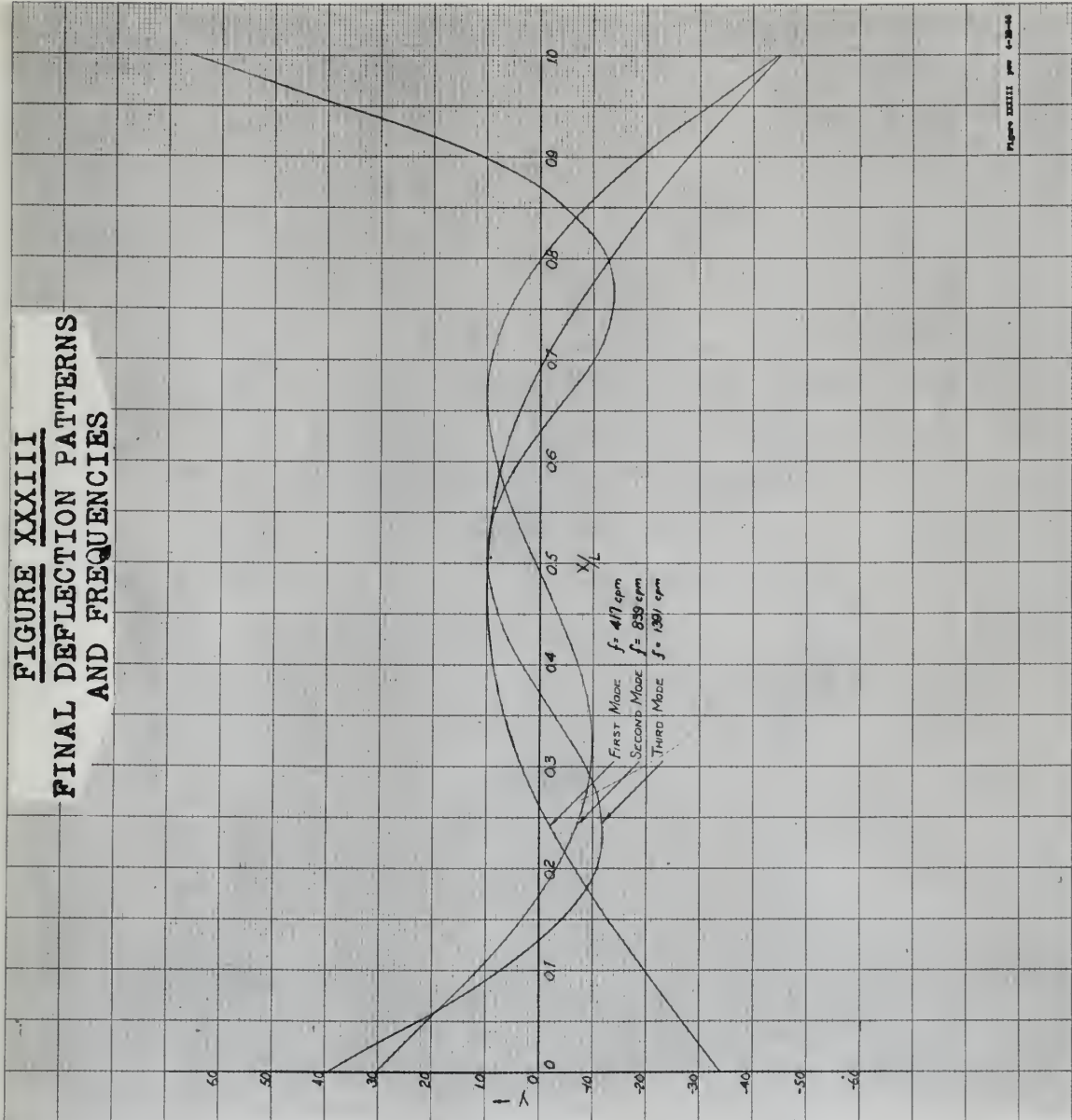


**FIGURE XXXII**  
**RESULTS OF ITERATION**  
**THIRD MODE**





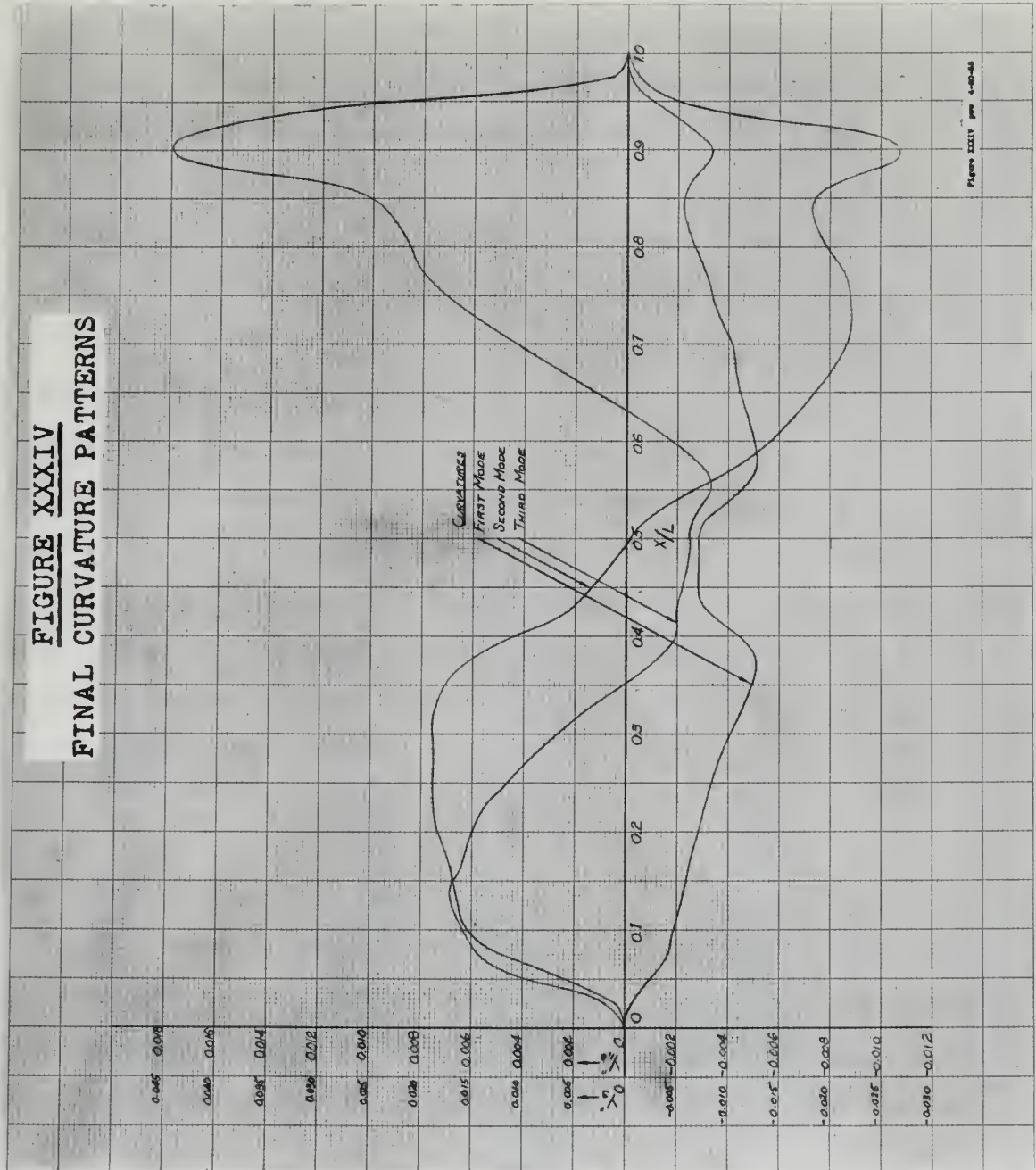
**FIGURE XXXIII**  
**FINAL DEFLECTION PATTERNS**  
**AND FREQUENCIES**







**FIGURE XXXIV**  
**FINAL CURVATURE PATTERNS**





APPENDIX CSample Calculation

This appendix contains a tabulation of the working functions for iteration, a table of constants used in the iteration, and a sample calculation in tabular form. All symbols are in accordance with those appearing in the Nomenclature.

The working functions (Table IV) pertain to all of the modal calculations. These functions were calculated as a matter of convenience as well as necessity. Their form is governed both by theory and by the conventions adopted and explained in Appendix A.

Table V contains a list of constants which, in general, apply to all three modal calculations. Orthogonality adjustment constants  $W_1$  and  $W_2$  apply to all modal calculations beyond the first mode and the second mode respectively. The derivation and explanation of the constants arising from integration may be found in Part I of Appendix A. The usual value of Young's Modulus was adopted, and the shear modulus was then computed from the well known relation:

$$G = \frac{E}{2(1 - \mu)}$$

where  $\mu$  is Poisson's Ratio, taken equal to 0.3.

The sample calculation embraces the third cycle of the third mode. This mode was chosen as being the most general of the three with respect to the orthogonality conditions. The third cycle of this mode was chosen for the calculations because it includes the rotatory inertia correction. (The second cycle does also, but the results thereof are less conclusive than those of the third cycle). The calculation is presented in tabular form in Table VI. The source of each function appears beneath its symbol at the head of the column. Amplifying remarks pertinent to the sample calculation appear at the end of Table VI.



## APPENDIX C

### Sample Calculation

This appendix contains a tabulation of the working functions for iteration, a table of constants used in the iteration, and a sample calculation in pocket form. All symbols are in accordance with those appearing in the nomenclature.

The working functions (Table IV) given in all of the model calculations. These functions were calculated as a matter of convenience as well as necessity. Their form is governed both by theory and by the conventions adopted and explained in Appendix A.

Table V contains a list of constants which, in general, apply to all three model calculations. Orthogonally adjusted constants  $W_1$  and  $W_2$  apply to all model calculations beyond the first mode and the second mode respectively. The derivation and explanation of the constants relating from integration may be found in Part I of Appendix A. The usual value of Young's modulus was adopted, and the shear modulus was then computed from the well known relation:

$$G = \frac{E}{2(1 + \mu)}$$

where  $\mu$  is Poisson's Ratio, taken equal to 0.3.

The sample calculation embraces the third cycle of the third mode. This mode was chosen as being the most general in the group with respect to the orthogonality conditions. The third cycle of this mode was chosen for the calculation because it includes the velocity inertia correction. (The second cycle does also, but the results thereof are less conclusive than those of the third cycle). The calculation is presented in pocket form in Table VI. The source of each function appears beneath its symbol at the head of the column. Any missing remarks pertinent to the sample calculation appear at the end of Table VI.

TABLE IV

WORKING FUNCTIONS FOR ITERATION

$x/L$	$mg$	$5mI_{zz}/L^2 A$	$KAG/10^6$	$10^6 L^2/EI_{zz}$	$(x-\bar{x})/L$
Units	lb/ft	lb/ft	lb	lb <sup>-1</sup>	-
0.000	400	0.4125	80.62	3.232	-0.472
.025	1270	.8506	126.10	3.232	.447
.050	2550	.9194	274.96	3.232	.422
.075	3800	.7839	798.56	2.020	.397
.100	4550	1.1824	644.78	1.212	.372
.125	5000	1.8079	649.66	0.795	.347
.150	5290	2.5176	725.87	0.570	.322
.175	5680	3.0802	801.80	0.432	.297
.200	6150	3.5065	951.07	0.359	.272
.225	6750	4.1259	1125.11	0.308	.247
.250	7450	4.8824	1207.72	0.276	.222
.275	8000	5.7269	1246.91	0.252	.197
.300	8450	6.2268	1266.08	0.242	.172
.325	8680	6.3097	1307.14	0.239	.147
.350	8800	6.2896	1331.19	0.236	.122
.375	8910	6.5440	1348.67	0.225	.097
.400	10200	9.1276	1399.02	0.186	.072
.425	11260	11.3521	1590.31	0.129	.047
.450	11850	11.4385	1567.78	0.110	-0.022
.475	11400	10.9989	1560.96	0.109	+0.003
.500	10400	10.0333	1727.01	0.110	.028
.525	9300	9.8506	1439.06	0.133	.053
.550	8880	8.2764	1349.89	0.186	.078
.575	8730	6.8408	1315.94	0.223	.103
.600	8700	6.5169	1302.26	0.235	.128
.625	8700	6.4157	1275.82	0.242	.153
.650	8650	6.1477	1249.20	0.255	.178
.675	8450	5.6035	1219.86	0.278	.203
.700	8000	4.9544	1186.64	0.303	.228
.725	7300	4.2214	1134.05	0.334	.253
.750	6400	3.4543	1100.75	0.373	.278
.775	5450	2.7277	918.46	0.425	.303
.800	4660	2.2035	832.70	0.485	.328
.825	3900	1.6861	718.31	0.592	.353
.850	3060	1.1393	805.30	0.808	.378
.875	2300	0.6257	681.92	1.469	.403
.900	1700	0.3511	318.94	2.851	.428
.925	1330	0.2564	240.69	4.041	.453
.950	1200	9.2427	213.88	4.848	.478
.975	1000	0.2438	226.03	4.848	.503
1.000	400	0.1100	184.00	4.848	+0.528



TABLE IV

RELATIONSHIP BETWEEN THE TWO VARIABLES

Variable	1947	1948	1949	1950	1951
1.000	0.000	0.000	0.000	0.000	0.000
0.999	0.001	0.001	0.001	0.001	0.001
0.998	0.002	0.002	0.002	0.002	0.002
0.997	0.003	0.003	0.003	0.003	0.003
0.996	0.004	0.004	0.004	0.004	0.004
0.995	0.005	0.005	0.005	0.005	0.005
0.994	0.006	0.006	0.006	0.006	0.006
0.993	0.007	0.007	0.007	0.007	0.007
0.992	0.008	0.008	0.008	0.008	0.008
0.991	0.009	0.009	0.009	0.009	0.009
0.990	0.010	0.010	0.010	0.010	0.010
0.989	0.011	0.011	0.011	0.011	0.011
0.988	0.012	0.012	0.012	0.012	0.012
0.987	0.013	0.013	0.013	0.013	0.013
0.986	0.014	0.014	0.014	0.014	0.014
0.985	0.015	0.015	0.015	0.015	0.015
0.984	0.016	0.016	0.016	0.016	0.016
0.983	0.017	0.017	0.017	0.017	0.017
0.982	0.018	0.018	0.018	0.018	0.018
0.981	0.019	0.019	0.019	0.019	0.019
0.980	0.020	0.020	0.020	0.020	0.020
0.979	0.021	0.021	0.021	0.021	0.021
0.978	0.022	0.022	0.022	0.022	0.022
0.977	0.023	0.023	0.023	0.023	0.023
0.976	0.024	0.024	0.024	0.024	0.024
0.975	0.025	0.025	0.025	0.025	0.025
0.974	0.026	0.026	0.026	0.026	0.026
0.973	0.027	0.027	0.027	0.027	0.027
0.972	0.028	0.028	0.028	0.028	0.028
0.971	0.029	0.029	0.029	0.029	0.029
0.970	0.030	0.030	0.030	0.030	0.030
0.969	0.031	0.031	0.031	0.031	0.031
0.968	0.032	0.032	0.032	0.032	0.032
0.967	0.033	0.033	0.033	0.033	0.033
0.966	0.034	0.034	0.034	0.034	0.034
0.965	0.035	0.035	0.035	0.035	0.035
0.964	0.036	0.036	0.036	0.036	0.036
0.963	0.037	0.037	0.037	0.037	0.037
0.962	0.038	0.038	0.038	0.038	0.038
0.961	0.039	0.039	0.039	0.039	0.039
0.960	0.040	0.040	0.040	0.040	0.040
0.959	0.041	0.041	0.041	0.041	0.041
0.958	0.042	0.042	0.042	0.042	0.042
0.957	0.043	0.043	0.043	0.043	0.043
0.956	0.044	0.044	0.044	0.044	0.044
0.955	0.045	0.045	0.045	0.045	0.045
0.954	0.046	0.046	0.046	0.046	0.046
0.953	0.047	0.047	0.047	0.047	0.047
0.952	0.048	0.048	0.048	0.048	0.048
0.951	0.049	0.049	0.049	0.049	0.049
0.950	0.050	0.050	0.050	0.050	0.050
0.949	0.051	0.051	0.051	0.051	0.051
0.948	0.052	0.052	0.052	0.052	0.052
0.947	0.053	0.053	0.053	0.053	0.053
0.946	0.054	0.054	0.054	0.054	0.054
0.945	0.055	0.055	0.055	0.055	0.055
0.944	0.056	0.056	0.056	0.056	0.056
0.943	0.057	0.057	0.057	0.057	0.057
0.942	0.058	0.058	0.058	0.058	0.058
0.941	0.059	0.059	0.059	0.059	0.059
0.940	0.060	0.060	0.060	0.060	0.060
0.939	0.061	0.061	0.061	0.061	0.061
0.938	0.062	0.062	0.062	0.062	0.062
0.937	0.063	0.063	0.063	0.063	0.063
0.936	0.064	0.064	0.064	0.064	0.064
0.935	0.065	0.065	0.065	0.065	0.065
0.934	0.066	0.066	0.066	0.066	0.066
0.933	0.067	0.067	0.067	0.067	0.067
0.932	0.068	0.068	0.068	0.068	0.068
0.931	0.069	0.069	0.069	0.069	0.069
0.930	0.070	0.070	0.070	0.070	0.070
0.929	0.071	0.071	0.071	0.071	0.071
0.928	0.072	0.072	0.072	0.072	0.072
0.927	0.073	0.073	0.073	0.073	0.073
0.926	0.074	0.074	0.074	0.074	0.074
0.925	0.075	0.075	0.075	0.075	0.075
0.924	0.076	0.076	0.076	0.076	0.076
0.923	0.077	0.077	0.077	0.077	0.077
0.922	0.078	0.078	0.078	0.078	0.078
0.921	0.079	0.079	0.079	0.079	0.079
0.920	0.080	0.080	0.080	0.080	0.080
0.919	0.081	0.081	0.081	0.081	0.081
0.918	0.082	0.082	0.082	0.082	0.082
0.917	0.083	0.083	0.083	0.083	0.083
0.916	0.084	0.084	0.084	0.084	0.084
0.915	0.085	0.085	0.085	0.085	0.085
0.914	0.086	0.086	0.086	0.086	0.086
0.913	0.087	0.087	0.087	0.087	0.087
0.912	0.088	0.088	0.088	0.088	0.088
0.911	0.089	0.089	0.089	0.089	0.089
0.910	0.090	0.090	0.090	0.090	0.090
0.909	0.091	0.091	0.091	0.091	0.091
0.908	0.092	0.092	0.092	0.092	0.092
0.907	0.093	0.093	0.093	0.093	0.093
0.906	0.094	0.094	0.094	0.094	0.094
0.905	0.095	0.095	0.095	0.095	0.095
0.904	0.096	0.096	0.096	0.096	0.096
0.903	0.097	0.097	0.097	0.097	0.097
0.902	0.098	0.098	0.098	0.098	0.098
0.901	0.099	0.099	0.099	0.099	0.099
0.900	0.100	0.100	0.100	0.100	0.100

TABLE IV (cont'd)

WORKING FUNCTIONS FOR ITERATION

$x/L$	$m_g(x-\bar{x})/L$	$E \int_0^x m(\xi-\bar{x}) d\xi / L^2$	$y_1$	$y_1 m_g$	$y_2$	$y_2 m_g$
Units	lb/ft	lb/ft	ft	lb	ft	lb
0.000	- 188.8	0	-3.408	-1363.2	+3.041	+1216.4
0.025	567.69	- 9.6	3.054	3878.58	2.578	3274.06
0.050	1076.1	30.6	2.695	6872.25	2.101	5357.55
0.075	1508.6	64.8	2.347	8918.6	1.609	6114.2
0.100	1692.6	103.2	1.994	9072.70	1.161	5282.55
0.125	1735.0	144.6	1.641	8205.0	0.726	3630.0
0.150	1703.38	185.4	1.299	6871.71	+ .336	+1777.44
0.175	1686.96	232.8	0.983	5611.84	- .018	- 107.24
0.200	1672.8	269.4	.682	4194.3	.307	1888.05
0.225	1667.25	311.4	.395	2666.25	.567	3827.25
0.250	1653.9	352.2	- .125	- 931.25	.762	5676.9
0.275	1576.0	391.2	+ .109	+ 872.0	.891	7128.0
0.300	1453.4	427.8	.325	2746.25	.963	8137.35
0.325	1275.96	460.8	.511	4435.48	.977	8480.36
0.350	1073.6	488.4	.661	5816.8	.940	8272.0
0.375	864.27	513.6	.788	7021.08	.845	7528.95
0.400	763.2	532.8	.878	9306.8	.706	7483.6
0.425	554.6	548.4	.944	11139.2	.532	6277.6
0.450	- 260.7	558.0	.987	11695.95	.336	3981.6
0.475	+ 34.2	561.6	1.000	11400.0	- .133	-1516.2
0.500	291.2	558.0	.989	10285.6	+ .085	+ 884.0
0.525	492.9	549.0	.963	8974.5	.302	2808.6
0.550	692.64	534.0	.912	8098.56	.519	4608.72
0.575	899.19	514.2	.835	7289.55	.693	6049.89
0.600	1113.6	489.0	.711	6185.7	.838	7290.6
0.625	1331.1	457.8	.568	4941.6	.938	8160.6
0.650	1539.7	423.0	.383	3312.95	.995	8606.75
0.675	1715.35	382.2	+ .169	+1428.05	.987	8340.15
0.700	1824.0	337.2	- .087	- 696.0	.920	7360.0
0.725	1846.9	289.8	.350	2555.0	.780	5694.0
0.750	1779.2	245.4	.654	4185.6	.582	3724.8
0.775	1651.35	203.4	.976	5319.2	+ .304	+1656.8
0.800	1528.48	163.2	1.310	6104.6	- .024	- 111.84
0.825	1376.7	126.6	1.662	6481.8	.411	1602.9
0.850	1156.68	94.8	2.032	6217.92	.849	2597.94
0.875	926.9	68.4	2.389	5494.7	1.337	3075.1
0.900	727.6	48.0	2.783	4731.1	1.883	3201.1
0.925	602.5	30.6	3.207	4265.3	2.503	3328.99
0.950	573.6	17.4	3.624	4348.8	3.174	3808.8
0.975	503.0	- 7.2	4.066	4126.0	3.808	3808.0
1.000	+ 211.2	0	-4.502	-1800.8	-4.450	-1780.0



1916	1915	1914	1913	1912	1911	1910	1909	1908	1907	1906	1905	1904	1903	1902	1901	1900	1899	1898	1897	1896	1895	1894	1893	1892	1891	1890	1889	1888	1887	1886	1885	1884	1883	1882	1881	1880	1879	1878	1877	1876	1875	1874	1873	1872	1871	1870	1869	1868	1867	1866	1865	1864	1863	1862	1861	1860	1859	1858	1857	1856	1855	1854	1853	1852	1851	1850	1849	1848	1847	1846	1845	1844	1843	1842	1841	1840	1839	1838	1837	1836	1835	1834	1833	1832	1831	1830	1829	1828	1827	1826	1825	1824	1823	1822	1821	1820	1819	1818	1817	1816	1815	1814	1813	1812	1811	1810	1809	1808	1807	1806	1805	1804	1803	1802	1801	1800	1799	1798	1797	1796	1795	1794	1793	1792	1791	1790	1789	1788	1787	1786	1785	1784	1783	1782	1781	1780	1779	1778	1777	1776	1775	1774	1773	1772	1771	1770	1769	1768	1767	1766	1765	1764	1763	1762	1761	1760	1759	1758	1757	1756	1755	1754	1753	1752	1751	1750	1749	1748	1747	1746	1745	1744	1743	1742	1741	1740	1739	1738	1737	1736	1735	1734	1733	1732	1731	1730	1729	1728	1727	1726	1725	1724	1723	1722	1721	1720	1719	1718	1717	1716	1715	1714	1713	1712	1711	1710	1709	1708	1707	1706	1705	1704	1703	1702	1701	1700	1699	1698	1697	1696	1695	1694	1693	1692	1691	1690	1689	1688	1687	1686	1685	1684	1683	1682	1681	1680	1679	1678	1677	1676	1675	1674	1673	1672	1671	1670	1669	1668	1667	1666	1665	1664	1663	1662	1661	1660	1659	1658	1657	1656	1655	1654	1653	1652	1651	1650	1649	1648	1647	1646	1645	1644	1643	1642	1641	1640	1639	1638	1637	1636	1635	1634	1633	1632	1631	1630	1629	1628	1627	1626	1625	1624	1623	1622	1621	1620	1619	1618	1617	1616	1615	1614	1613	1612	1611	1610	1609	1608	1607	1606	1605	1604	1603	1602	1601	1600	1599	1598	1597	1596	1595	1594	1593	1592	1591	1590	1589	1588	1587	1586	1585	1584	1583	1582	1581	1580	1579	1578	1577	1576	1575	1574	1573	1572	1571	1570	1569	1568	1567	1566	1565	1564	1563	1562	1561	1560	1559	1558	1557	1556	1555	1554	1553	1552	1551	1550	1549	1548	1547	1546	1545	1544	1543	1542	1541	1540	1539	1538	1537	1536	1535	1534	1533	1532	1531	1530	1529	1528	1527	1526	1525	1524	1523	1522	1521	1520	1519	1518	1517	1516	1515	1514	1513	1512	1511	1510	1509	1508	1507	1506	1505	1504	1503	1502	1501	1500	1499	1498	1497	1496	1495	1494	1493	1492	1491	1490	1489	1488	1487	1486	1485	1484	1483	1482	1481	1480	1479	1478	1477	1476	1475	1474	1473	1472	1471	1470	1469	1468	1467	1466	1465	1464	1463	1462	1461	1460	1459	1458	1457	1456	1455	1454	1453	1452	1451	1450	1449	1448	1447	1446	1445	1444	1443	1442	1441	1440	1439	1438	1437	1436	1435	1434	1433	1432	1431	1430	1429	1428	1427	1426	1425	1424	1423	1422	1421	1420	1419	1418	1417	1416	1415	1414	1413	1412	1411	1410	1409	1408	1407	1406	1405	1404	1403	1402	1401	1400	1399	1398	1397	1396	1395	1394	1393	1392	1391	1390	1389	1388	1387	1386	1385	1384	1383	1382	1381	1380	1379	1378	1377	1376	1375	1374	1373	1372	1371	1370	1369	1368	1367	1366	1365	1364	1363	1362	1361	1360	1359	1358	1357	1356	1355	1354	1353	1352	1351	1350	1349	1348	1347	1346	1345	1344	1343	1342	1341	1340	1339	1338	1337	1336	1335	1334	1333	1332	1331	1330	1329	1328	1327	1326	1325	1324	1323	1322	1321	1320	1319	1318	1317	1316	1315	1314	1313	1312	1311	1310	1309	1308	1307	1306	1305	1304	1303	1302	1301	1300	1299	1298	1297	1296	1295	1294	1293	1292	1291	1290	1289	1288	1287	1286	1285	1284	1283	1282	1281	1280	1279	1278	1277	1276	1275	1274	1273	1272	1271	1270	1269	1268	1267	1266	1265	1264	1263	1262	1261	1260	1259	1258	1257	1256	1255	1254	1253	1252	1251	1250	1249	1248	1247	1246	1245	1244	1243	1242	1241	1240	1239	1238	1237	1236	1235	1234	1233	1232	1231	1230	1229	1228	1227	1226	1225	1224	1223	1222	1221	1220	1219	1218	1217	1216	1215	1214	1213	1212	1211	1210	1209	1208	1207	1206	1205	1204	1203	1202	1201	1200	1199	1198	1197	1196	1195	1194	1193	1192	1191	1190	1189	1188	1187	1186	1185	1184	1183	1182	1181	1180	1179	1178	1177	1176	1175	1174	1173	1172	1171	1170	1169	1168	1167	1166	1165	1164	1163	1162	1161	1160	1159	1158	1157	1156	1155	1154	1153	1152	1151	1150	1149	1148	1147	1146	1145	1144	1143	1142	1141	1140	1139	1138	1137	1136	1135	1134	1133	1132	1131	1130	1129	1128	1127	1126	1125	1124	1123	1122	1121	1120	1119	1118	1117	1116	1115	1114	1113	1112	1111	1110	1109	1108	1107	1106	1105	1104	1103	1102	1101	1100	1099	1098	1097	1096	1095	1094	1093	1092	1091	1090	1089	1088	1087	1086	1085	1084	1083	1082	1081	1080	1079	1078	1077	1076	1075	1074	1073	1072	1071	1070	1069	1068	1067	1066	1065	1064	1063	1062	1061	1060	1059	1058	1057	1056	1055	1054	1053	1052	1051	1050	1049	1048	1047	1046	1045	1044	1043	1042	1041	1040	1039	1038	1037	1036	1035	1034	1033	1032	1031	1030	1029	1028	1027	1026	1025	1024	1023	1022	1021	1020	1019	1018	1017	1016	1015	1014	1013	1012	1011	1010	1009	1008	1007	1006	1005	1004	1003	1002	1001	1000	999	998	997	996	995	994	993	992	991	990	989	988	987	986	985	984	983	982	981	980	979	978	977	976	975	974	973	972	971	970	969	968	967	966	965	964	963	962	961	960	959	958	957	956	955	954	953	952	951	950	949	948	947	946	945	944	943	942	941	940	939	938	937	936	935	934	933	932	931	930	929	928	927	926	925	924	923	922	921	920	919	918	917	916	915	914	913	912	911	910	909	908	907	906	905	904	903	902	901	900	899	898	897	896	895	894	893	892	891	890	889	888	887	886	885	884	883	882	881	880	879	878	877	876	875	874	873	872	871	870	869	868	867	866	865	864	863	862	861	860	859	858	857	856	855	854	853	852	851	850	849	848	847	846	845	844	843	842	841	840	839	838	837	836	835	834	833	832	831	830	829	828	827	826	825	824	823	822	821	820	819	818	817	816	815	814	813	812	811	810	809	808	807	806	805	804	803	802	801	800	799	798	797	796	795	794	793	792	791	790	789	788	787	786	785	784	783	782	781	780	779	778	777	776	775	774	773	772	771	770	769	768	767	766	765	764	763	762	761	760	759	758	757	756	755	754	753	752	751	750	749	748	747	746	745	744	743	742	741	740	739	738	737	736	735	734	733	732	731	730	729	728	727	726	725	724	723	722	721	720	719	718	717	716	715	714	713	712	711	710	709	708	707	706	705	704	703	702	701	700	699	698	697	696	695	694	693	692	691	690	689	688	687	686	685	684	683	682	681	680	679	678	677	676	675	674	673	672	671	670	669	668	667	666	665	664	663	662	661	660	659	658	657	656	655	654	653	652	651	650	649	648	647	646	645	644	643	642	641	640	639	638	637	636	635	634	633	632	631	630	629	628	627	626	625	624	623	622	621	620	619	618	617	616	615	614	613	612	611	610	609	608	607	606	605	604	603	602	601	600	599	598	597	596	595	594	593	592	591	590	589	588	587	586	585	584	583	582	581	580	579	578	577	576	575	574	573	572	571	570	569	568	567	566	565	564	563	562	561	560	559	558	557	556	555	554	553	552	551	550	549	548	547	546	545	544	543	542	541	540	539	538	537	536	535	534	533	532	531	530	529	528	527	526	525	524	523	522	521	520	519	518	517	516	515	514	513	512	511	510	509	508	507	506	505	504	503	502	501	500	499	498	497	496	495	494	493	492	491	490	489	488	487	486	485	484
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TABLE V

CONSTANTS REQUIRED FOR ITERATION

CONSTANT	SYMBOL	VALUE
$\int_0^L mg \frac{dx}{L}$	$\Delta/L$	6450 lb/ft
$\int_0^L \int_0^x \frac{(\xi - \bar{x})}{L} mg \frac{d\xi}{L} \frac{dx}{L}$	P	-287.64 lb/ft
$\int_0^L \frac{(x - \bar{x})^2}{L^2} mg \frac{dx}{L}$	N	300 lb/ft
$\int_0^L y_1^2 mg \frac{dx}{L}$	$W_1$	8062.5 ft-lb
$\int_0^L y_2^2 mg \frac{dx}{L}$	$W_2$	5040.0 ft-lb
Length of model	L	100.5 ft
Longitudinal centroid (from bow)	$\bar{x}$	(0.472) L ft.
Shear Modulus	G	$11.5 \times 10^6$ lb/in. <sup>2</sup>
Young's Modulus	E	$30.0 \times 10^6$ lb/in. <sup>2</sup>

COMPLAINT RECEIVED FOR THE YEAR

[illegible]



TABLE VI

- SAMPLE CALCULATION -

THIRD CYCLE OF THIRD MODE

(1)	(2)	(3)	(4)	(5)	(6)
$x/L$	$\gamma_{3-2}$	$g_{3-2}$	$g_{3-2}/L$	$\gamma_{3-2} \frac{g_{3-2}}{L}$	$g_{3-2}/L$
Source	Second Cycle 3rd mode	$-(2)mgw^2$	$\int_x (3) dx/L$	See Remarks	(4) + (5)
0.000	+3.8865	-1554.60	0	-15	- 15
0.025	3.0105	3823.34	- 66	31	97
0.050	2.1546	5494.23	189	33	222
0.075	1.3490	5126.20	330	25	355
0.100	.6706	3051.23	444	33	477
0.125	+ .0522	- 261.	480	42	522
0.150	- .4299	+2274.17	450	47	497
0.175	.8467	4809.26	360	43	403
0.200	1.0802	6643.23	213	35	248
0.225	1.1644	7859.7	- 24	25	- 49
0.250	1.1507	8572.72	+183	-13	+170
0.275	1.0320	8256.	393	+ 2	395
0.300	.8174	6907.03	573	18	591
0.325	.5519	4790.49	720	28	748
0.350	- .2616	+2302.08	804	32	834
0.375	+ .0626	- 557.77	825	33	858
0.400	.3536	3748.16	777	43	820
0.425	.6171	7281.78	651	41	692
0.450	.8304	9840.24	417	28	445
0.475	.9498	10827.72	+147	+13	+160
0.500	1.0000	10400.	-129	- 3	-132
0.525	.9641	8966.13	366	17	383
0.550	.8545	7587.96	579	30	609
0.575	.6660	5814.18	753	37	790
0.600	.3894	3387.78	867	43	910
0.625	+ .0385	- 334.95	915	44	959
0.650	- .3607	+3120.06	879	40	919
0.675	.6882	5815.29	765	27	792
0.700	1.0052	8041.6	591	-11	602
0.725	1.2355	9019.15	351	+ 5	346
0.750	1.3653	8737.92	-132	15	-117
0.775	1.3738	7487.21	+ 63	27	+ 90
0.800	1.2433	5793.78	222	34	256
0.825	.9713	3788.07	339	35	374
0.850	- .5199	+1590.89	396	30	426
0.875	+ .0874	- 201.02	405	21	426
0.900	.9876	1678.92	381	16	397
0.925	2.1970	2922.01	321	14	335
0.950	3.5343	4241.16	228	15	243
0.975	5.0216	5021.60	+108	16	124
1.000	+6.4208	-2568.32	0	+ 7	+ 7



TABLE I

- SUMMARY OF DATA -

THESE DATA ARE FROM THE

(1)	(2)	(3)	(4)	(5)	(6)
Y	X	Y	X	Y	X
(1) + (2)	(3) + (4)	(5) + (6)	(1) - (2)	(3) - (4)	(5) - (6)
1.000	0.000	0.000	0.000	0.000	0.000
0.999	0.001	0.001	0.001	0.001	0.001
0.998	0.002	0.002	0.002	0.002	0.002
0.997	0.003	0.003	0.003	0.003	0.003
0.996	0.004	0.004	0.004	0.004	0.004
0.995	0.005	0.005	0.005	0.005	0.005
0.994	0.006	0.006	0.006	0.006	0.006
0.993	0.007	0.007	0.007	0.007	0.007
0.992	0.008	0.008	0.008	0.008	0.008
0.991	0.009	0.009	0.009	0.009	0.009
0.990	0.010	0.010	0.010	0.010	0.010
0.989	0.011	0.011	0.011	0.011	0.011
0.988	0.012	0.012	0.012	0.012	0.012
0.987	0.013	0.013	0.013	0.013	0.013
0.986	0.014	0.014	0.014	0.014	0.014
0.985	0.015	0.015	0.015	0.015	0.015
0.984	0.016	0.016	0.016	0.016	0.016
0.983	0.017	0.017	0.017	0.017	0.017
0.982	0.018	0.018	0.018	0.018	0.018
0.981	0.019	0.019	0.019	0.019	0.019
0.980	0.020	0.020	0.020	0.020	0.020
0.979	0.021	0.021	0.021	0.021	0.021
0.978	0.022	0.022	0.022	0.022	0.022
0.977	0.023	0.023	0.023	0.023	0.023
0.976	0.024	0.024	0.024	0.024	0.024
0.975	0.025	0.025	0.025	0.025	0.025
0.974	0.026	0.026	0.026	0.026	0.026
0.973	0.027	0.027	0.027	0.027	0.027
0.972	0.028	0.028	0.028	0.028	0.028
0.971	0.029	0.029	0.029	0.029	0.029
0.970	0.030	0.030	0.030	0.030	0.030
0.969	0.031	0.031	0.031	0.031	0.031
0.968	0.032	0.032	0.032	0.032	0.032
0.967	0.033	0.033	0.033	0.033	0.033
0.966	0.034	0.034	0.034	0.034	0.034
0.965	0.035	0.035	0.035	0.035	0.035
0.964	0.036	0.036	0.036	0.036	0.036
0.963	0.037	0.037	0.037	0.037	0.037
0.962	0.038	0.038	0.038	0.038	0.038
0.961	0.039	0.039	0.039	0.039	0.039
0.960	0.040	0.040	0.040	0.040	0.040
0.959	0.041	0.041	0.041	0.041	0.041
0.958	0.042	0.042	0.042	0.042	0.042
0.957	0.043	0.043	0.043	0.043	0.043
0.956	0.044	0.044	0.044	0.044	0.044
0.955	0.045	0.045	0.045	0.045	0.045
0.954	0.046	0.046	0.046	0.046	0.046
0.953	0.047	0.047	0.047	0.047	0.047
0.952	0.048	0.048	0.048	0.048	0.048
0.951	0.049	0.049	0.049	0.049	0.049
0.950	0.050	0.050	0.050	0.050	0.050
0.949	0.051	0.051	0.051	0.051	0.051
0.948	0.052	0.052	0.052	0.052	0.052
0.947	0.053	0.053	0.053	0.053	0.053
0.946	0.054	0.054	0.054	0.054	0.054
0.945	0.055	0.055	0.055	0.055	0.055
0.944	0.056	0.056	0.056	0.056	0.056
0.943	0.057	0.057	0.057	0.057	0.057
0.942	0.058	0.058	0.058	0.058	0.058
0.941	0.059	0.059	0.059	0.059	0.059
0.940	0.060	0.060	0.060	0.060	0.060
0.939	0.061	0.061	0.061	0.061	0.061
0.938	0.062	0.062	0.062	0.062	0.062
0.937	0.063	0.063	0.063	0.063	0.063
0.936	0.064	0.064	0.064	0.064	0.064
0.935	0.065	0.065	0.065	0.065	0.065
0.934	0.066	0.066	0.066	0.066	0.066
0.933	0.067	0.067	0.067	0.067	0.067
0.932	0.068	0.068	0.068	0.068	0.068
0.931	0.069	0.069	0.069	0.069	0.069
0.930	0.070	0.070	0.070	0.070	0.070
0.929	0.071	0.071	0.071	0.071	0.071
0.928	0.072	0.072	0.072	0.072	0.072
0.927	0.073	0.073	0.073	0.073	0.073
0.926	0.074	0.074	0.074	0.074	0.074
0.925	0.075	0.075	0.075	0.075	0.075
0.924	0.076	0.076	0.076	0.076	0.076
0.923	0.077	0.077	0.077	0.077	0.077
0.922	0.078	0.078	0.078	0.078	0.078
0.921	0.079	0.079	0.079	0.079	0.079
0.920	0.080	0.080	0.080	0.080	0.080
0.919	0.081	0.081	0.081	0.081	0.081
0.918	0.082	0.082	0.082	0.082	0.082
0.917	0.083	0.083	0.083	0.083	0.083
0.916	0.084	0.084	0.084	0.084	0.084
0.915	0.085	0.085	0.085	0.085	0.085
0.914	0.086	0.086	0.086	0.086	0.086
0.913	0.087	0.087	0.087	0.087	0.087
0.912	0.088	0.088	0.088	0.088	0.088
0.911	0.089	0.089	0.089	0.089	0.089
0.910	0.090	0.090	0.090	0.090	0.090
0.909	0.091	0.091	0.091	0.091	0.091
0.908	0.092	0.092	0.092	0.092	0.092
0.907	0.093	0.093	0.093	0.093	0.093
0.906	0.094	0.094	0.094	0.094	0.094
0.905	0.095	0.095	0.095	0.095	0.095
0.904	0.096	0.096	0.096	0.096	0.096
0.903	0.097	0.097	0.097	0.097	0.097
0.902	0.098	0.098	0.098	0.098	0.098
0.901	0.099	0.099	0.099	0.099	0.099
0.900	0.100	0.100	0.100	0.100	0.100

TABLE VI (cont'd)

(1)	(7)	(8)	(9)	(10)	(11)
$x/L$	$gN/L^2$	$\epsilon \delta'_{3-3} 10^6$	$\epsilon \bar{\delta}_{3-3} \frac{10^6}{L}$	$\epsilon \beta \frac{10^6}{L}$	$\epsilon \eta' \frac{10^6}{L}$
Source	$\int_x^{(6)} dx/L$	(7). $\frac{10^6 L}{EI}$	$\int_x^{(8)} dx/L$	(4). $\frac{10^6}{KAG}$	(9)+(10)
0.000	0	0	0	0	0
0.025	- 0.90	+ 2.909	+ 0.015	-0.5234	- 0.5084
0.050	4.50	14.544	.255	0.6874	- 0.4324
0.075	10.80	21.816	.735	0.4132	+ 0.3218
0.100	19.80	23.998	1.335	0.6886	0.6464
0.125	32.40	25.758	1.980	0.7388	1.2412
0.150	44.55	25.394	2.655	0.6199	2.0351
0.175	55.35	23.911	3.300	0.4490	2.8510
0.200	63.45	22.779	3.915	0.2240	3.6910
0.225	68.40	21.067	4.500	-0.0213	4.4787
0.250	63.45	17.512	5.010	+0.1515	5.1615
0.275	56.25	14.175	5.445	0.3152	5.7602
0.300	42.75	10.346	5.790	0.4526	6.2426
0.325	24.75	5.915	6.030	0.5508	6.5808
0.350	- 3.15	+ 0.743	6.120	0.6040	6.7240
0.375	+18.90	- 4.252	6.120	0.6117	6.7317
0.400	39.60	7.366	6.000	0.5554	6.5554
0.425	60.75	7.837	5.820	0.4094	6.2294
0.450	75.15	8.266	5.640	0.2660	5.9060
0.475	83.25	9.074	5.445	+0.0942	5.5392
0.500	84.60	9.306	5.220	-0.0747	5.1453
0.525	78.30	10.414	4.980	0.2543	4.7257
0.550	66.60	12.388	4.710	0.4289	4.2811
0.575	49.50	11.038	4.425	0.5722	3.8528
0.600	28.80	6.768	4.200	0.6658	3.5342
0.625	+ 5.40	- 1.307	4.125	0.7172	3.4078
0.650	-18.00	+ 4.590	4.155	0.7036	3.4514
0.675	37.80	10.508	4.350	0.6271	3.7229
0.700	54.45	16.498	4.725	0.4980	4.2270
0.725	66.60	22.244	5.235	0.3095	4.9255
0.750	72.00	26.856	5.850	-0.1199	5.7301
0.775	72.00	30.600	6.585	+0.0686	6.6536
0.800	66.60	32.301	7.410	0.2666	7.6766
0.825	58.50	34.632	8.415	0.4719	8.8869
0.850	47.25	38.178	9.195	0.4917	9.6867
0.875	36.00	52.884	10.335	0.5939	10.9289
0.900	23.85	67.996	11.880	1.1946	13.0746
0.925	14.85	60.009	13.560	1.3337	14.8937
0.950	6.75	32.724	14.835	1.0660	15.9010
0.975	- 0.45	+ 2.182	15.180	+0.4778	15.6578
1.000	0	0	+15.225	0	+15.225



[illegible]

TABLE VI (cont'd)

(1)	(12)	(13)	(14)	(15)	(16)
$x/L$	$g^2 \eta' \cdot \frac{10^6}{L} \int_0^x \frac{(\xi-x)}{L} \frac{d\xi}{L}$	$g \gamma'_{3-3} \cdot \frac{10^6}{L}$	$g \gamma \cdot \frac{10^6}{L^2}$	$10^6 g^2 \gamma \frac{m}{L^2}$	$g \bar{\gamma}_{3-3} \cdot \frac{10^6}{L^2}$
Source	$(11) \int_0^x g^m (\frac{\xi-x}{L}) \frac{d\xi}{L}$	$(11) + C_3 g \frac{10^6}{L}$	$\int_x (13) \frac{dx}{L}$	(14) mg	$(14) + \frac{C_4 g 10^6}{L^2}$
0.000	0	- 5.3539	0	0	+0.8023
0.025	+ 4.881	5.8623	-0.1372	- 174.2	0.6651
0.050	+ 13.231	5.7863	0.2850	728.8	0.5165
0.075	- 20.853	5.0321	0.4208	1599.0	0.3815
0.100	66.708	4.7075	0.5422	2467.0	0.2601
0.125	179.478	4.1127	0.6525	3262.5	0.1498
0.150	377.308	3.3188	0.7470	3951.6	+0.0553
0.175	663.713	2.5029	0.8212	4664.4	-0.0189
0.200	994.355	1.6629	0.8775	5396.6	0.0752
0.225	1394.667	0.8752	0.9090	6135.8	0.1067
0.250	1817.880	- 0.1924	0.9225	6872.6	0.1202
0.275	2253.390	+ 0.4063	0.9248	7398.4	0.1225
0.300	2670.584	0.8887	0.9090	7681.1	0.1067
0.325	3032.433	1.2269	0.8842	7674.9	0.0819
0.350	3284.002	1.3701	0.8528	7504.6	0.0505
0.375	3448.156	1.3778	0.8168	7277.7	-0.0145
0.400	3492.717	1.2015	0.7852	6923.1	+0.0171
0.425	3416.203	0.8755	0.7560	6920.8	0.0463
0.450	3295.548	0.5521	0.7380	6745.3	0.0643
0.475	3110.815	+ 0.1853	0.7268	6285.5	0.0755
0.500	2871.077	- 0.2086	0.7245	7534.8	0.0778
0.525	2594.409	0.6282	0.7346	6831.8	0.0677*
0.550	2286.107	1.0728	0.7549	6703.5	0.0474*
0.575	1981.110	1.5011	0.7875	6874.9	+0.0148
0.600	1728.224	1.8197	0.8258	7184.5	-0.0235
0.625	1560.091	1.9461	0.8730	7595.1	0.0707
0.650	1459.942	1.9025	0.9202	7959.7	0.1179
0.675	1422.892	1.6310	0.9675	8175.4	0.1652
0.700	1425.344	1.1269	1.0058	8046.4	0.2035
0.725	1427.41	- 0.4284	1.0238	7473.7	0.2215
0.750	1406.167	+ 0.3762	1.0260	6566.4	0.2237
0.775	1353.342	1.2997	1.0058	5481.6	0.2035
0.800	1252.821	2.3227	0.9652	4497.8	0.1629
0.825	1125.082	3.5330	0.8888	3466.3	-0.0865
0.850	918.299	4.3328	0.7898	2416.8	+0.0125
0.875	747.537	5.5750	0.6638	1526.7	0.1385
0.900	627.581	7.7207	0.4972	845.2	0.3051
0.925	455.747	9.5398	0.2722	362.0	0.5301
0.950	276.677	10.5471	-0.0225	- 27.	0.7798
0.975	- 112.736	10.3039	+0.2362	+ 236.2	1.0385
1.000	0	+ 9.8711	+0.4860	+ 194.4	+1.2883



[illegible]

TABLE VI (cont'd)

(1)	(17)	(18)	(19)	(20)	(21)
$x/L$	$10^6 g^2 \bar{y}_{3.3} m/L^2$	$\frac{10^6 g^2 \bar{y}_{3.3} m(x-\bar{x})}{L^2}$	$\frac{10^6 g^2 \bar{y}_{3.3} y_1 m}{L^2}$	$\frac{10^6 g^2 \bar{y}_{3.3} y_2 m}{L^2}$	$C_i y_i$
Source	(16) · mg	(16) · mg $\frac{(x-\bar{x})}{L}$	(16) · y · mg	(16) · y <sub>2</sub> mg	See Remarks
0.000	+ 320.9	-151.5	-1093.7	+ 975.9	-0.2133
0.025	844.7	377.6	2579.64	2177.6	.1912
0.050	1317.1	555.8	3549.5	2767.2	.1687
0.075	1449.7	575.5	3402.4	2332.6	.1469
0.100	1183.5	440.2	2359.8	1374.0	.1248
0.125	749.	259.9	1229.1	543.8	.1027
0.150	+ 292.5	- 94.2	- 580.0	+ 98.3	.0813
0.175	- 107.4	+ 31.9	+ 106.1	+ 1.9	.0618
0.200	462.5	125.8	315.4	142.0	.0427
0.225	720.2	177.9	284.5	408.4	.0247
0.250	895.5	198.8	+ 111.9	682.4	- .0078
0.275	980.0	193.1	- 106.8	873.2	+ .0068
0.300	901.6	155.1	293.0	868.3	.0203
0.325	710.9	104.5	363.3	694.5	.0320
0.350	444.4	54.2	293.7	417.7	.0414
0.375	- 129.2	+ 12.5	- 101.8	+ 109.2	.0493
0.400	+ 181.3	- 13.1	+ 159.1	- 128.0	.0550
0.425	546.3	- 25.7	515.7	- 290.7	.0591
0.450	762.0	- 16.8	752.0	- 256.0	.0618
0.475	860.7	+ 2.6	860.7	- 114.5	.0626
0.500	809.1	22.7	800.2	+ 68.8	.0619
0.525	629.6	33.4	607.6	+ 190.1	.0604
0.550	420.9	32.8	383.9	+ 218.5	.0571
0.575	+ 129.2	+ 13.3	+ 107.9	+ 89.5	.0523
0.600	- 204.5	- 26.2	- 145.4	- 171.3	.0445
0.625	615.1	94.1	349.4	577.0	.0356
0.650	1019.8	181.5	390.6	1014.7	.0240
0.675	1395.9	283.4	- 235.9	1377.8	+ .0106
0.700	1628.0	371.2	+ 141.6	1497.7	- .0054
0.725	1617.0	409.1	565.6	1261.2	.0219
0.750	1431.7	398.0	936.3	833.2	.0409
0.775	1109.1	336.0	1082.5	- 337.2	.0611
0.800	759.1	249.0	994.4	+ 18.2	.0820
0.825	- 337.4	-119.1	+ 560.7	+ 138.70	.1040
0.850	+ 38.3	+ 14.5	- 77.7	- 32.5	.1272
0.875	318.6	128.4	761.0	425.9	.1496
0.900	518.7	222.0	1443.5	976.7	.1742
0.925	705.0	319.4	2261.0	1764.7	.2008
0.950	935.8	447.3	3391.2	2970.1	.2269
0.975	1038.5	522.4	4284.9	3954.6	.2583
1.000	+ 515.3	+272.1	-2320.0	-2293.2	- .2818



TABLE IV (continued)

(1)	(2)	(3)	(4)	(5)	(6)
$\frac{1}{n} \sum_{i=1}^n x_i^2$	$\frac{1}{n} \sum_{i=1}^n x_i$	$\frac{1}{n} \sum_{i=1}^n x_i^2$	$\frac{1}{n} \sum_{i=1}^n x_i$	$\frac{1}{n} \sum_{i=1}^n x_i^2$	$\frac{1}{n} \sum_{i=1}^n x_i$
Sample	(1) - (2)	(1) - (2)	(1) - (2)	Sample	(1) - (2)
0.000	+ 0.000	- 0.000	- 0.000	0.000	+ 0.000
0.001	+ 0.001	- 0.001	- 0.001	0.001	+ 0.001
0.002	+ 0.002	- 0.002	- 0.002	0.002	+ 0.002
0.003	+ 0.003	- 0.003	- 0.003	0.003	+ 0.003
0.004	+ 0.004	- 0.004	- 0.004	0.004	+ 0.004
0.005	+ 0.005	- 0.005	- 0.005	0.005	+ 0.005
0.006	+ 0.006	- 0.006	- 0.006	0.006	+ 0.006
0.007	+ 0.007	- 0.007	- 0.007	0.007	+ 0.007
0.008	+ 0.008	- 0.008	- 0.008	0.008	+ 0.008
0.009	+ 0.009	- 0.009	- 0.009	0.009	+ 0.009
0.010	+ 0.010	- 0.010	- 0.010	0.010	+ 0.010
0.011	+ 0.011	- 0.011	- 0.011	0.011	+ 0.011
0.012	+ 0.012	- 0.012	- 0.012	0.012	+ 0.012
0.013	+ 0.013	- 0.013	- 0.013	0.013	+ 0.013
0.014	+ 0.014	- 0.014	- 0.014	0.014	+ 0.014
0.015	+ 0.015	- 0.015	- 0.015	0.015	+ 0.015
0.016	+ 0.016	- 0.016	- 0.016	0.016	+ 0.016
0.017	+ 0.017	- 0.017	- 0.017	0.017	+ 0.017
0.018	+ 0.018	- 0.018	- 0.018	0.018	+ 0.018
0.019	+ 0.019	- 0.019	- 0.019	0.019	+ 0.019
0.020	+ 0.020	- 0.020	- 0.020	0.020	+ 0.020
0.021	+ 0.021	- 0.021	- 0.021	0.021	+ 0.021
0.022	+ 0.022	- 0.022	- 0.022	0.022	+ 0.022
0.023	+ 0.023	- 0.023	- 0.023	0.023	+ 0.023
0.024	+ 0.024	- 0.024	- 0.024	0.024	+ 0.024
0.025	+ 0.025	- 0.025	- 0.025	0.025	+ 0.025
0.026	+ 0.026	- 0.026	- 0.026	0.026	+ 0.026
0.027	+ 0.027	- 0.027	- 0.027	0.027	+ 0.027
0.028	+ 0.028	- 0.028	- 0.028	0.028	+ 0.028
0.029	+ 0.029	- 0.029	- 0.029	0.029	+ 0.029
0.030	+ 0.030	- 0.030	- 0.030	0.030	+ 0.030
0.031	+ 0.031	- 0.031	- 0.031	0.031	+ 0.031
0.032	+ 0.032	- 0.032	- 0.032	0.032	+ 0.032
0.033	+ 0.033	- 0.033	- 0.033	0.033	+ 0.033
0.034	+ 0.034	- 0.034	- 0.034	0.034	+ 0.034
0.035	+ 0.035	- 0.035	- 0.035	0.035	+ 0.035
0.036	+ 0.036	- 0.036	- 0.036	0.036	+ 0.036
0.037	+ 0.037	- 0.037	- 0.037	0.037	+ 0.037
0.038	+ 0.038	- 0.038	- 0.038	0.038	+ 0.038
0.039	+ 0.039	- 0.039	- 0.039	0.039	+ 0.039
0.040	+ 0.040	- 0.040	- 0.040	0.040	+ 0.040
0.041	+ 0.041	- 0.041	- 0.041	0.041	+ 0.041
0.042	+ 0.042	- 0.042	- 0.042	0.042	+ 0.042
0.043	+ 0.043	- 0.043	- 0.043	0.043	+ 0.043
0.044	+ 0.044	- 0.044	- 0.044	0.044	+ 0.044
0.045	+ 0.045	- 0.045	- 0.045	0.045	+ 0.045
0.046	+ 0.046	- 0.046	- 0.046	0.046	+ 0.046
0.047	+ 0.047	- 0.047	- 0.047	0.047	+ 0.047
0.048	+ 0.048	- 0.048	- 0.048	0.048	+ 0.048
0.049	+ 0.049	- 0.049	- 0.049	0.049	+ 0.049
0.050	+ 0.050	- 0.050	- 0.050	0.050	+ 0.050

TABLE VI (cont'd)

(1)	(22)	(23)	(24)	(25)	(26)
$x/L$	$0.11\gamma_2$	$\bar{B}(x-\bar{x})$	$10^6 \gamma_{3-3}/L^2$	$\gamma_{3-3n}$	$L\sigma_{3-3n}$
Source	See Remarks	See Remarks	(16) + $\bar{A}$ + (21) + (21) + (23)	(24) $\gamma_{3-3}^* \frac{10^6 g}{L^2}$	(13) - $\gamma_{3-3}^* \frac{10^6 g}{L^2}$
0.000	+0.0675	-0.0705	+ .5899	+3.9248	-35.622
0.025	.0572	.0667	.4683	3.1158	35.522
0.050	.0466	.0630	.3353	2.2309	33.925
0.075	.0357	.0593	.2149	1.4298	30.731
0.100	.0258	.0555	.1095	0.7285	26.739
0.125	.0161	.0518	+ .0153	+0.1018	22.448
0.150	+ .0075	.0481	- .0627	-0.4172	17.957
0.175	- .0004	.0443	.1215	0.8084	13.665
0.200	.0068	.0406	.1614	1.0739	9.574
0.225	.0126	.0369	.1770	1.1777	5.681
0.250	.0169	.0331	.1741	1.1584	- 2.288
0.275	.0198	.0294	.1610	1.0712	+ 0.606
0.300	.0214	.0257	.1296	0.8623	2.902
0.325	.0217	.0219	.0896	0.5961	4.498
0.350	.0209	.0182	- .0443	-0.2947	5.097
0.375	.0188	.0145	+ .0054	+0.0359	5.097
0.400	.0157	.0107	.0496	0.3300	4.299
0.425	.0118	.0070	.0905	0.6021	3.101
0.450	.0075	- .0033	.1192	0.7931	1.904
0.475	- .0030	+ .0004	.1394	0.9275	+ 0.606
0.500	+ .0019	.0042	.1497	0.9960	- 0.891
0.525	.0067	.0079	.1466	0.9754	2.488
0.550	.0015	.0116	.1315	0.8749	4.284
0.575	.0154	.0154	.1018	0.6773	6.180
0.600	.0186	.0191	.0626	0.4165	7.677
0.625	.0208	.0228	+ .0124	+0.0825	8.176
0.650	.0221	.0266	- .0413	-0.2748	7.977
0.675	.0219	.0303	.0985	0.6554	6.679
0.700	.0204	.0340	.1506	1.0020	4.184
0.725	.0173	.0378	.1844	1.2269	- 0.791
0.750	.0129	.0415	.2063	1.3726	+ 3.301
0.775	+ .0067	.0452	.2088	1.3892	8.191
0.800	- .0005	.0490	.1925	1.2808	13.680
0.825	.0091	.0527	.1430	0.9514	20.367
0.850	.0188	.0564	- .0732	-0.4870	25.556
0.875	.0297	.0602	+ .0233	+0.1550	33.141
0.900	.0418	.0639	.1569	1.0439	43.421
0.925	.0556	.0676	.3452	2.2967	54.598
0.950	.0705	.0714	.5577	3.7106	63.082
0.975	.0845	.0751	.7747	5.1544	65.377
1.000	- .0988	+ .0788	+ .9904	+ 6.5895	+ 65.676



[illegible]

TABLE VI (Continued)

AMPLIFYING REMARKS RELATIVE TO SAMPLE CALCULATION

<u>COLUMN</u>	<u>REMARKS</u>
3 -----	Inertia loading with $\omega^2$ taken equal to 1.
5 -----	$\gamma_{3-2}$ is the normalized component of slope due to bending, taken from the previous cycle.
5, 6 -----	$\gamma_{3-2} \frac{8mI}{LA}$ is the derivative with respect to $x$ of the moment of the inertia forces, at any section, about the vertical centroidal axis. When $\gamma_{3-2} \frac{8mI}{LA}$ is added to the shear force, one obtains $dM/dx$ , which reflects the effect of lateral inertia forces plus shear due to rotatory inertia within the approximation imposed by using $\gamma$ from the previous cycle.
9 -----	Component of slope due to bending moment before determination of its true axis.
10 -----	Component of slope due to shear flexibility.
11 -----	Column 9 plus column 10. Slope before true axis determination.
12 -----	$\int_0^L 8\gamma' \frac{10^6}{L} \int_0^x 8^m \frac{(\xi - \bar{x})}{L} \frac{d\xi}{L} \frac{dx}{L} = -1540$ $C_3 \left( 8 \frac{10^6}{L} \right) = - \frac{\int_0^L 8\gamma' \frac{10^6}{L} \int_0^x 8^m \frac{(\xi - \bar{x})}{L} \frac{d\xi}{L} \frac{dx}{L}}{P}$ $= - \frac{-1540}{-287.64} = -5.3539$

TABLE VI (Continued)

AMPLITUDE MEMBER RELATIVE TO SAMPLE CALCULATION

REMARKS

COLUMN

1. For the leading wave  $\omega$  taken equal to 1.

2.  $\chi_{-2}$  is the normalized component of slope due to

leading wave taken from the previous cycle.

3.  $\chi_{-2} \frac{8mI}{14}$  is the derivative with respect to  $x$  of

the moment of the leading wave, at any section.

about the vertical centroidal axis. When  $\chi_{-2} \frac{8mI}{14}$

is added to the shear force, one obtains  $SM_{-2}$

which reflects the effect of lateral inertia forces

give shear due to rotary inertia within the

approximation imposed by using  $\chi$  from the previous cycle.

4. Component of slope due to leading moment balance

disturbance of the wave axis.

5. Component of slope due to shear flexibility.

6. Column 5 plus column 4. Slope balance law axis

disturbance.

$$12. \quad \frac{\int_0^1 \left( \frac{8mI}{14} \right) \chi_{-2} dx}{\int_0^1 \left( \frac{8mI}{14} \right) \chi_{-2} dx} = -1.20$$

$$\frac{\int_0^1 \left( \frac{8mI}{14} \right) \chi_{-2} dx}{\int_0^1 \left( \frac{8mI}{14} \right) \chi_{-2} dx} = -1.20$$

$$-1.20 = \frac{-1.20}{1.00}$$



TABLE VI Continued

COLUMNREMARKS

15

$$\int_0^L \frac{10^6 \eta g^2 m}{L^2} \frac{dx}{L} = -5175$$

$$C_4 \left( \frac{8 \cdot 10^6}{L^2} \right) = - \frac{\int_0^L \frac{10^6 \eta g^2 m}{L^2} \frac{dx}{L}}{\Delta / L}$$

$$= - \frac{-5175}{6450} = +0.8023$$

17, 18, 19, 20

Calculations necessary for determination of adjustment constants to be applied to  $\frac{10^6 g \bar{y}_{3-3}}{L^2}$  in order that the resultant deflection pattern is orthogonal with respect to:

1. Rigid body translation
2. Rigid body rotation
3. First mode deflection pattern
4. Second mode deflection pattern

17

1. Orthogonality with respect to rigid body translation:

Condition:  $\int_0^L y_3 m dx = 0$

Result:

$$\int_0^L \frac{10^6 g^2 \bar{y}_{3-3} m dx}{L^2} = -25.2$$

$$\bar{A} = - \frac{\int_0^L \frac{10^6}{L^2} g^2 \bar{y}_{3-3} m \frac{dx}{L}}{\Delta / L}$$

$$= - \frac{-25.2}{6450} = +0.0039$$



TABLE VI Continued

COLUMN

12

REMARKS

$$\frac{10^6}{L^2} \frac{m}{L} = -0.173$$

$$\frac{10^6}{L^2} \frac{m}{L} = -0.173$$

$$\frac{-0.173}{0.450} = +0.382$$

17, 18, 19, 20

Calculations necessary for determination of adjustment constants to be applied to  $\frac{10^6}{L^2} \frac{m}{L}$  in order that the residual deflection pattern is orthogonal with respect to:

1. rigid body translation
2. rigid body rotation
3. first mode deflection pattern
4. second mode deflection pattern

21

Orthogonality with respect to rigid body translation:

$$\int_0^L \frac{1}{2} m dx = 0$$

Result:

$$\frac{10^6}{L^2} \frac{m}{L} = -0.173$$

$$\frac{10^6}{L^2} \frac{m}{L} = -0.173$$

$$\frac{-0.173}{0.450} = +0.382$$

TABLE VI Continued

COLUMNREMARKS

18

2. Orthogonality with respect to rigid body rotation

$$\text{Condition: } \int_0^L \bar{y}_3 m g (x - \bar{x}) dx = 0$$

Result:

$$\int_0^L \frac{10^6}{L^2} g^2 \bar{y}_{3.3} m \frac{(x - \bar{x})}{L^3} \frac{dx}{L} = -44.8$$

$$\bar{B}(L) = - \frac{\int_0^L \frac{10^6}{L^2} g^2 \bar{y}_{3.3} m \frac{(x - \bar{x})}{L^3} \frac{dx}{L}}{N}$$

$$= - \frac{-44.8}{300} = + 0.1493$$

19

3. Orthogonality with respect to 1st mode:

$$\text{Condition: } \int_0^L \bar{y}_1 \bar{y}_3 m dx = 0$$

$$\text{Result: } \int_0^L \frac{10^6}{L^2} g^2 \bar{y}_1 \bar{y}_{3.3} m \frac{dx}{L} = -505$$

$$\frac{10^6}{L^2} (C_1) = - \frac{\int_0^L \frac{10^6}{L^2} g^2 \bar{y}_1 \bar{y}_{3.3} m \frac{dx}{L}}{W_1}$$

$$= - \frac{-505}{8062.5} = + 0.0626$$

20

4. Orthogonality with respect to 2nd mode

$$\text{Condition: } \int_0^L \bar{y}_2 \bar{y}_3 m dx = 0$$

TABLE VI  
Continued

TERMS

COLUMNS

3. Orthogonality with respect to rigid body rotation

18

Condition:  $\int_0^1 \bar{Y}_3 \cos(\pi - \pi) d\pi = 0$

Result:

$$\int_0^1 \frac{10^6 \bar{Y}_3^2 m}{\bar{Y}_3^2} d\pi = 10.0$$

$$\frac{\int_0^1 10^6 \bar{Y}_3^2 m \frac{(\pi - \pi)}{\pi^2} d\pi}{\int_0^1 \frac{10^6 \bar{Y}_3^2 m}{\bar{Y}_3^2} d\pi} = 0$$

$$\frac{-10.0}{100} = -0.100$$

4. Orthogonality with respect to the modes

19

Condition:  $\int_0^1 \bar{Y}_3 \cos \pi d\pi = 0$

$$\text{Result: } \int_0^1 \frac{10^6 \bar{Y}_3^2 m}{\bar{Y}_3^2} d\pi = 10.0$$

$$\frac{\int_0^1 10^6 \bar{Y}_3^2 m \frac{\pi \cos \pi}{\pi^2} d\pi}{\int_0^1 \frac{10^6 \bar{Y}_3^2 m}{\bar{Y}_3^2} d\pi} = 0$$

$$\frac{-10.0}{100} = -0.100$$

5. Orthogonality with respect to the modes

20

Condition:  $\int_0^1 \bar{Y}_3 \cos \pi d\pi = 0$

TABLE VI Continued

COLUMNREMARKS

20

$$\text{Result: } \int_0^L \frac{10^6 g^2}{L^2} \bar{y}_{3-3} y_2 m \frac{dx}{L} = -112$$

$$C_{11} = - \frac{\int_0^L \frac{10^6 g^2}{L^2} \bar{y}_{3-3} y_2 m \frac{dx}{L}}{W_2}$$

$$= - \frac{-112}{5940} = + 0.0222$$

21

Adjustment to be applied to  $\frac{10^6 g}{L^2} \bar{y}_{3-3}$  for orthogonality with first mode deflection pattern.

22

Adjustment to be applied to  $\frac{10^6 g}{L^2} \bar{y}_{3-3}$  for orthogonality with second mode deflection pattern

23

Adjustment to be applied to  $\frac{10^6 g}{L^2} \bar{y}_{3-3}$  for orthogonality with rigid body rotation.

24

$$\frac{10^6 g y_{3-3}}{L^2} = \frac{10^6 g \bar{y}_{3-3}}{L^2} + \bar{A} + \bar{B}(x-\bar{x}) + C_1 y_1 + C_{11} y_2$$

The value of  $\frac{10^6 g}{L^2} y_{3-3}$  at the positive anti-node



(near  $X/L = 0.5$ )

is the normalizing value and also a function of the eigenvalue. Its value is 0.1503 for this cycle, i.e.,

$$\frac{10^6 g}{L^2} y_{3-3}^* = 0.1503$$





TABLE VI ContinuedCOLUMNREMARKS

25

Normalized resultant deflection pattern,  $y_{3-3,n}$ 

$$y_{3-3,n} = \frac{10^6 g \bar{y}_{3-3}}{L^2 (0.1503)}$$

26

Normalization of  $\gamma_{3-3}$  for use in next cycle.CALCULATION OF FREQUENCY

$$\omega_{3-3}^2 = \frac{1}{y_{3-3}^*} = \frac{10^6 g / L^2}{\frac{10^6 g y_{3-3}^*}{L^2}} = \frac{10^6 \cdot 32.2}{(0.1503)(100.5)^2}$$

$$\omega_{3-3}^2 = \frac{3168.025}{.1503} = 21,211$$

$$\omega_{3-3} = \underline{145.64 \text{ rad/sec}}$$

$$T_{3-3} = \frac{2\pi}{\omega_{3-3}} = \frac{6.28318}{145.64} \times 1000 = \underline{43.14 \text{ Milli-seconds}}$$

$$f_{3-3} = \omega_{3-3} \frac{60}{2\pi} = \frac{145.64 \cdot 60}{6.28318} = 1391 \text{ c.p.m.}$$

TABLE VI Continued

TABLE VI

COLUMN

Normalized constant deflection pattern,  $\gamma_{3-3}$

$$\gamma_{3-3} = \frac{10^6 \times 1.2}{1 \times (0.1003)} = 1.2 \times 10^7$$

Maximization of  $\gamma_{3-3}$  for one cycle

CALCULATION OF FREQUENCY

$$\omega_{3-3} = \frac{1}{\gamma_{3-3}} = \frac{1}{1.2 \times 10^7} = 8.33 \times 10^{-8} \text{ sec}$$

$$\omega_{3-3} = \frac{1}{1.2 \times 10^7} = 8.33 \times 10^{-8} \text{ sec}$$

$$\omega_{3-3} = 1.42 \times 10^4 \text{ rad/sec}$$

$$T_{3-3} = \frac{1}{\omega_{3-3}} = \frac{1}{1.42 \times 10^4} = 7.04 \times 10^{-5} \text{ sec}$$

$$\omega_{3-3} = \frac{1}{T_{3-3}} = \frac{1}{7.04 \times 10^{-5}} = 1.42 \times 10^4 \text{ rad/sec}$$

APPENDIX D - SUPPLEMENTARY DISCUSSION1. Secondary Effects Not Treated:

**Shear Lag:** The classical beam vibrations as applied to ships do not take special notice of the fact that resistance to deflection is concentrated generally at the extremes of the cross section and that, in horizontal bending, stress transmitted from the deck to the side will not maintain a constant level down the side as indicated by the constant distance from the neutral axis. This is because of the effect known as shear lag. While important in lightly framed ships where decks and longitudinal bulkheads play little part structurally, it was not thought to be significant in the ring-framed model of this investigation. Shear lag is treated in detail by R. A. Anderson and J. C. Houbolt ("The Effect of Shear Lag in the Bending Vibrations of Box Beams", NACA TN No. 1583, May 1948) and others.

**Second Frequency Spectrum:** The simple Harmonic assumption applied to Timoshenko's complete mechanism leads to an equation which is fourth order in circular frequency, indicating the possibility of a second real value of frequency for a given mode. Traill-Nash and Collar [63] were able to determine this second spectrum of frequencies for certain special cases of uniform beams. Its physical importance was not determined, but it was thought to arise from a resonant interaction between rotatory inertia forces and shear stiffness.

The iterative method of this paper, while considering both effects, and permitting them to interact in the sequence, is based on a single assumed frequency, and does not facilitate simultaneous interaction. Hence the second frequency spectrum is not explicitly noted.

**Torsional Coupling:** It should be noted that coupling can occur between torsional and horizontal flexural vibration, with resultant effects on the patterns and frequencies of each. Such coupling has not been treated in this investigation.



# APPENDIX II - SUPPLEMENTARY DISCUSSION

## 1. Geometry of the Specimen

Figure 1. The specimen being studied is applied to a plate. The specimen is the first specimen in the series. It is concentrated generally at the surface of the specimen and that in horizontal bending, stress transmitted from the back to the side will not maintain a constant level down the side as indicated by the constant distance from the neutral axis. This is because of the effect of the shear lag. While important in highly stressed areas where shear and longitudinal stresses are high, it is not important, it was not thought to be significant in the long-term study of this investigation. The lag is noted in detail by A. A. Johnson and J. C. Johnson, "The Effect of Shear Lag in the Bending of Plates", NACA, TN No. 1252, May 1948, and others.

Second Frequency Spectrum. The single harmonic spectrum applied to Timoshenko's complete mechanical model is an equation which is too simple in character to represent the possibility of a second real value of frequency for a given mode. Timoshenko and Cotter [1] were able to determine this second spectrum of frequencies for certain special cases of certain cases. The physical significance was not determined, but it was thought to arise from a permanent interaction between primary inertia forces and shear stresses.

The present method of this paper, while considering both effects, and assuming them to interact in the sequence, is based on a single assumed frequency, and does not facilitate simultaneous interaction. Hence the second frequency spectrum is not explicitly noted.

Totaling Coupling. It should be noted that coupling can occur between torsional and horizontal bending vibration with torsional effects on the pattern and frequency of shear. Such coupling can be noted in this investigation.

## 2. More Exact Orthogonality Formulation:

A review of the theoretical development of this method revealed that the formulation of the orthogonality relationships used were inexact when applied to orthogonalizing with non-trivial modes when rotatory inertia is considered. The effect of the discrepancy is discussed in the next subsection. The development of the more exact formulation\* will now be given.

The normalized deflection and bending slope, and the circular frequency of each of two different modes are given by

$$y_i, \delta_i, \omega_i, \quad y_j, \delta_j, \omega_j.$$

Multiplying equations (25) and (27) of Appendix A-1 by  $\delta_j$  and  $y_j$  respectively, and integrating, we have for mode  $i$ ,

$$\int_0^L \delta_j (EI \delta_i')' dx + \int_0^L \frac{Im}{A} \omega_i^2 \delta_i \delta_j dx + \int_0^L KAG (\gamma_i' \delta_j - \delta_i \gamma_j) dx = 0 \quad (69a)$$

$$\int_0^L m \gamma_i \gamma_j \omega_i^2 dx + \int_0^L [KAG (\gamma_i' - \delta_i)]' \gamma_j dx = 0 \quad (69b)$$

and using the multipliers from mode  $i$ , we have for mode  $j$ ,

$$\int_0^L \delta_i (EI \delta_j')' dx + \int_0^L \frac{Im}{A} \omega_j^2 \delta_i \delta_j dx + \int_0^L KAG (\gamma_j' \delta_i - \delta_j \delta_i) dx = 0 \quad (70a)$$

$$\int_0^L m \gamma_i \gamma_j \omega_j^2 dx + \int_0^L [KAG (\gamma_j' - \delta_j)]' \gamma_i dx = 0 \quad (70b)$$

Integrating the first term of (69a) by parts, we have

$$EI \delta_j \delta_i' \Big|_0^L - \int_0^L EI \delta_i' \delta_j' dx + \int_0^L \frac{Im}{A} \omega_i^2 \delta_i \delta_j dx + \int_0^L KAG (\gamma_i' \delta_j - \delta_i \gamma_j) dx = 0 \quad (71)$$

\*Due to Prof. S. H. Crandall, Massachusetts Institute of Technology and to C. L. Dolph, Willow Run Research Center, University of Michigan

# I. More Exact Orthogonality Formulation

A review of the theoretical development of this method revealed that the formulation of the orthogonality relationships used were incorrect when applied to waveguiding with non-crystal modes when isotropy (anisotropy is considered). The effect of the anisotropy is discussed in the next subsection. The development of the more exact formulation will now be given.

The normalized dispersion and loading slope, and the characteristic frequency of each of the system modes are given by

$$\gamma_i, \omega_i, \gamma_i', \omega_i', \gamma_i'', \omega_i''$$

utilizing equations (12) and (13) or (14) and (15) respectively, and integrating, we have for mode  $i$ ,

$$\int_0^L \gamma_i' \gamma_i'' dx + \int_0^L \frac{1}{\omega_i'} \omega_i'' \gamma_i' \gamma_i'' dx + \int_0^L KAO(\gamma_i' \gamma_i'' - \gamma_i'' \gamma_i') dx = 0 \quad (16a)$$

$$\int_0^L \gamma_i' \gamma_i'' \omega_i' dx + \int_0^L \left[ \gamma_i' (\gamma_i'' - \gamma_i''') \right] \gamma_i' dx = 0 \quad (16b)$$

and using the relationships from mode  $j$ , we have for mode  $j$ ,

$$\int_0^L \gamma_j' \gamma_j'' dx + \int_0^L \frac{1}{\omega_j'} \omega_j'' \gamma_j' \gamma_j'' dx + \int_0^L KAO(\gamma_j' \gamma_j'' - \gamma_j'' \gamma_j') dx = 0 \quad (17a)$$

$$\int_0^L \gamma_j' \gamma_j'' \omega_j' dx + \int_0^L \left[ \gamma_j' (\gamma_j'' - \gamma_j''') \right] \gamma_j' dx = 0 \quad (17b)$$

Integrating the first term of (16a) by parts, we have

$$\int_0^L \gamma_i' \gamma_i'' dx = \left[ \gamma_i' \gamma_i'' \right]_0^L - \int_0^L \gamma_i'' \gamma_i' dx + \int_0^L \frac{1}{\omega_i'} \omega_i'' \gamma_i' \gamma_i'' dx + \int_0^L KAO(\gamma_i' \gamma_i'' - \gamma_i'' \gamma_i') dx = 0 \quad (17)$$

\*Due to Prof. R. M. Gossard, Massachusetts Institute of Technology and to C. L. Dolph, Bell Telephone Laboratories, University of Michigan



where the first term vanishes since curvature in a free-free beam vanishes at both ends. The corresponding operation is carried out on (70a), and the result subtracted from (71), giving

$$(\omega_i^2 - \omega_j^2) \int_0^L \frac{Im}{A} \gamma_i \gamma_j dx + \int_0^L KAG [\gamma_i' \gamma_j - \gamma_j' \gamma_i] dx = 0 \quad (72)$$

Now (69b) is rewritten, integrating the second term by parts, giving

$$\int_0^L m \gamma_i \gamma_j \omega_i^2 dx + KAG (\gamma_i' - \gamma_i) \gamma_j \Big|_0^L - \int_0^L KAG (\gamma_i' \gamma_j' - \gamma_i \gamma_j') dx = 0 \quad (73)$$

where the second term vanishes, since  $KAG (\gamma_i' - \gamma_i)$  is identical to shear, which must vanish at both ends of a free-free beam. The corresponding operation is carried out for (70b), and the resulting equation is subtracted from (73), giving

$$(\omega_i^2 - \omega_j^2) \int_0^L m \gamma_i \gamma_j dx + \int_0^L KAG [\gamma_i \gamma_j' - \gamma_j \gamma_i'] dx = 0 \quad (74)$$

Adding (72) and (74) gives the orthogonality relationship

$$(\omega_i^2 - \omega_j^2) \int_0^L \left( \frac{Im}{A} \gamma_i \gamma_j + m \gamma_i \gamma_j \right) dx = 0 \quad (75)$$

which, for  $\omega_i \neq \omega_j$ , leaves the useful relationship

$$\int_0^L \left( \frac{Im}{A} \gamma_i \gamma_j + m \gamma_i \gamma_j \right) dx = 0 \quad (76)$$

This should be compared with the relationship used in the investigation

$$\int_0^L (m \gamma_i \gamma_j) dx = 0 \quad (77)$$

which is seen to be in error by the integral of the first term in (76).

The importance of this term varies from mode to mode, and must be evaluated for higher modes to determine the necessity of applying the exact condition. It does not lend itself readily to application in the iterative procedure of this thesis, since it provides corrections



where the first term contains also convective in a free-free beam

variables at both ends. The corresponding equation is written out as

(106), and the result suggested from (71), giving

$$(107) \quad \int_0^L (\omega_i^2 - \omega_j^2) \left( \frac{1}{\omega_i^2} \right) dx + \int_0^L \frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right) dx = 0 \quad (107)$$

Now (107) is rewritten, integrating the second term by parts, giving

$$(108) \quad \int_0^L \frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right) dx + \int_0^L \frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right) dx = 0 \quad (108)$$

where the second term vanishes, since  $\frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right)$  is identical to

mean, which must vanish at both ends of a free-free beam. The corresponding

equation is written out for (106), and the resulting equation is

suggested from (71), giving

$$(109) \quad \int_0^L (\omega_i^2 - \omega_j^2) \left( \frac{1}{\omega_i^2} \right) dx + \int_0^L \frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right) dx = 0 \quad (109)$$

Adding (107) and (109) gives the orthogonality relationship

$$(110) \quad \int_0^L (\omega_i^2 - \omega_j^2) \left( \frac{1}{\omega_i^2} \right) dx + \int_0^L \frac{1}{\omega_i^2} \left( \frac{1}{\omega_j^2} \right) dx = 0 \quad (110)$$

which, for  $\omega_i \neq \omega_j$ , leaves the useful relationship

$$(111) \quad \int_0^L (\omega_i^2 - \omega_j^2) \left( \frac{1}{\omega_i^2} \right) dx = 0 \quad (111)$$

This should be compared with the relationship used in the investigation

$$(112) \quad \int_0^L (\omega_i^2 - \omega_j^2) \left( \frac{1}{\omega_i^2} \right) dx = 0 \quad (112)$$

which is seen to be in error by the integral of the first term in (10).

The importance of this term arises from mode to mode, and

must be evaluated for higher modes to determine the necessity of

applying the exact condition. It does not lend itself readily to application

in the iterative procedure of this thesis, since it provides corrections

to bending slope and deflection, quantities which become available at different stages of the process, and which cannot be corrected separately. An acceptable, though somewhat unwieldy approach would be to apply the complete check at the deflection level, examining the slope correction to see if its magnitude warrants its use. If it is significant, the bending slope is corrected, and the new total slope integrated. The new deflection curve is given the exact orthogonality check, this time assuming that the slope needs no correction.

In the case of this thesis, an auxiliary investigation showed that the complete check changed the natural frequency for the third modes (previously corrected by the approximate relationship) by 0.36%, and that the effect was even smaller on the second mode frequency. It appears that for the modes of interest, the added inconvenience of the full correction may have but little justification. Where applied, the correction is stated by the following equations:

$$(\gamma_j)_c = \gamma_j + C_1 \gamma_1 + C_{11} \gamma_{11} \quad (78)$$

$$(\gamma_j)_c = \gamma_j + C_1 \gamma_1 + C_{11} \gamma_{11} \quad (79)$$

where the constant for each prior mode is defined by:

$$C_1 = - \frac{\int_0^L \frac{mI}{A} \gamma_1 \gamma_j dx + \int_0^L m \gamma_1 \gamma_j dx}{\int_0^L \frac{mI}{A} \gamma_1^2 dx + \int_0^L m \gamma_1^2 dx} \quad (80)$$

### 3. Third Mode Frequency:

As noted in the Discussion of Results, the natural frequency obtained for the third mode (1391 cpm) by the iterative process and orthogonality filtering is considered to be slightly high. This section

in computing slope and deflection, quantities which become available at different stages of the process, and which cannot be obtained separately. An acceptable, though somewhat haphazard approach would be to apply the complete chart at the deflection level, assuming the slope correction to be 0. The resulting deflection is then used to compute the new deflection curve is given the exact orthogonality check, this time assuming that the slope needs no correction.

In the case of this problem, an auxiliary investigation showed that the complete chart changed the natural frequency by 0.002, and (approximately) corrected the approximate relationship by 0.002, and that the effect was even smaller on the second mode frequency. It appears that for the modes considered, the added importance of the full correction may have been little justification. Where applied, the correction is given by the following equations:

$$(73) \quad \gamma_i = \gamma_i + C_{11} \gamma_i + C_{12} \gamma_i$$

$$(74) \quad \delta_i = \delta_i + C_{11} \delta_i + C_{12} \delta_i$$

where the constant for each point made is defined by:

$$(75) \quad C_1 = \frac{\int_0^L m I \delta_i \delta_i dx + \int_0^L m \gamma_i \gamma_i dx}{\int_0^L m I \gamma_i \gamma_i dx + \int_0^L m \delta_i \delta_i dx}$$

### 3. Third Mode Frequency:

As noted in the introduction, the natural frequency obtained for the third mode (13.6 cps) by the iterative process and orthogonality fitting is considered to be slightly high. This problem



presents estimates of the correct value obtained from several independent approaches.

a. The cycle-by-cycle behavior of frequencies in the third mode calculation manifest; alternation casting some doubt on the value from the third cycle. Consideration of this behavior seems to indicate ultimate convergence to a value somewhere between the second and third cycle results, whereas earlier modes showed continually falling trends. The change in behavior in the third mode appears to stem from a starting assumption which was in error, even after orthogonalization, to a greater degree than were those of the other modes. This may be due in part to the use of the inexact orthogonality check, as well as to any of the sources of error listed in the Discussion of Results. We conclude from this approach that the third mode natural frequency should be in the neighborhood of 1385 to 1390 cps.

b. The more exact orthogonality check formulated in equation (80) was applied to the result of the last iteration of the third mode, disregarding the effect on slope. The constants became

$$C_i = - \frac{-505 - 17}{8062.5 + 57.8} = + 0.06428$$

instead of

$$C_i = - \frac{-505}{8062.5} = + 0.0626$$

and

$$C_{ii} = - \frac{-112 + 1.2}{5040 + 76.3} = + 0.02166$$

instead of

$$C_{ii} = - \frac{-112}{5040} = + 0.0222$$

The net effect was to lower the natural frequency from 1391 to 1386 cpm. The change in mode shape was insignificant.





c. The empirical ratio of natural frequencies observed for a number of ships, 1: 2: 3, although approximate, and not verified for submarines, is applied for interest. It results in no significant change in the second mode, but depresses the third mode frequency to 1251 cpm. This is clearly much lower than there is reason to expect, but tends to verify the suspicion that 1391 is perhaps high.

d. Table VII lists frequencies for the model under consideration, obtained theoretically. Tabulated with the values obtained formally in this investigation (MIT) are those obtained by David Taylor Model Basin (DTMB) by analog computer and those obtained by the authors at Underwater Explosion Research Division (UERD), both sets including shear flexibility but not rotatory inertia.

TABLE VII  
CALCULATED MODEL FREQUENCIES (cpm)

Mode	MIT	DTMB	UERD
1	417	444	428
2	838	888	857
3	1391	1392	-

Since the added refinement of rotatory inertia should tend to lower the frequencies of this thesis as compared to the other sets, it again appears that 1391 cpm is too high for the third mode, and that perhaps a reasonable value to expect would be 1320 cpm. This is based on both the ratio series for the DTMB data and the suspected trend of equal or greater reduction with mode number due to rotatory inertia.

It should be pointed out that neither set of comparison frequencies is corroborated, and experimental frequencies have not yet been obtained for the model under consideration.

e. Considering all of the above factors, it can best be concluded that the third mode frequency is  $1355 \pm 2.6\%$ .

c. The empirical ratio of natural frequencies measured for a  
 particular rotor,  $f_{\text{rotor}}$ , to  $f_{\text{shaft}}$  is always approximate, and not verified for  
 comparison, is applied for interest. It results in an approximate change  
 in the second mode, but decreases the third mode frequency to 1251  
 rpm. This is likely much lower than there is reason to expect, and  
 tends to verify the suspicion that 1251 is perhaps high.

d. Table VII lists frequencies for the 1-0-0-0 rotor considered.  
 Now, obtained independently. Comparison with the values obtained previously  
 in this investigation (1911) and those obtained by David Taylor Model  
 Basin (DTMB) by using computer and those obtained by the authors at  
 University of California Research Division (UCRD), also were including  
 these frequencies for not rotating shafts.

TABLE VII

CALCULATED MODES - FREQUENCIES (rpm)

Mode	1911	DTMB	UCRD
1	417	444	444
2	838	888	857
3	1251	1252	-

Since the added adjustment of rotating shafts should tend to lower the  
 frequencies of this rotor as compared to the other rotor, it again appears  
 that 1251 rpm is too high for the third mode, and that perhaps a reasonable  
 value to expect would be 1325 rpm. This is called as both the rotor rotor  
 for the DTMB data and the suggested trend in regard to greater reduction  
 with mode number due to rotating shafts.

It should be pointed out that neither set of comparison frequencies  
 is corroborated, and experimental frequencies have not been obtained  
 for the rotor under consideration.

4. Considering all of the above factors, it can well be concluded  
 that the third mode frequency is 1251 rpm.



APPENDIX E - STRUCTURAL DATA

Table VIII contains a tabulation of structural data necessary for the modal calculations. The source of these data is the Underwater Explosions Research Division of the Norfolk Naval Shipyard, under whose auspices the model submarine referred to in this work was constructed.

The mass appearing in Table VIII includes the structural mass of the submarine, the mass of her ballast water, the mass of her solid contents, and the virtual water mass.

The virtual water mass was calculated from the following formula:

$$m_v = \rho \pi b^2 \quad (81)$$

where  $b$  is half the depth of the submarine structure at any section, including the protuberance of the rudder and conning tower where applicable, and  $\rho$  = density of water in lb/ft<sup>3</sup>.

Equation (81) is based on the assumption that the cross section is elliptical with the major axis congruent with the vertical center line of the submarine. The wash of the water through the superstructure is neglected. The resulting curve of virtual water mass was further modified by an empirical correction explained in David Taylor Model Basin Report # 632. This correction consisted of multiplying the  $m_v$  curve by 0.83 over most of its length, with a more severe correction at the ends. Only one-third of the additional virtual water mass contributed by the rudder and the conning tower was used.

These corrections, ~~are~~ somewhat arbitrary, as all estimates of virtual water mass tend to be. The total empirical correction causes only a very slight change in the ship vibration behavior. No experimental data that verifies the virtual water mass distribution is available for this submarine.

The ends of the total mass curve used by the authors were altered



# APPENDIX B

Table VII contains a tabulation of structural data necessary for

the needed calculations. The source of these data is the Underwater  
Explorations Research Division of the Norfolk Naval Shipyard, under whose  
auspices the model information related to this work was constructed.  
The mass appearing in Table VIII includes the structural mass  
of the submarine, the mass of her ballast water, the mass of her solids  
contents, and the virtual water mass.  
The virtual water mass was calculated from the following

formula:

$$m_v = \rho \pi b^2 L \quad (81)$$

where  $b$  is half the depth of the submarine structure at any section,  
including the protrusion of the rudder and conning tower where appli-  
cable, and  $\rho$  = density of water is 1.025.  
Equation (81) is based on the assumption that the cross section  
is elliptical with the major axis congruent with the vertical center line  
of the submarine. The wash of the water through the superstructure is  
neglected. The resulting curve of virtual water mass was further modi-  
fied by an empirical correction explained in David Taylor Model Basin  
Report # 632. This correction consisted of multiplying the  $m_v$  curve  
by 0.83 over most of its length, with a more severe correction at the  
ends. Only one-third of the additional virtual water mass contributed  
by the rudder and the conning tower was used.  
These corrections, as somewhat arbitrary, as all estimates of  
virtual water mass tend to be. The total empirical correction causes  
only a very slight change in the ship vibration behavior. No experi-  
mental data that verified the virtual water mass distribution is available  
for this submarine.  
The ends of the total mass curve used by the authors were fitted

slightly so that the ordinates did not approach zero rapidly, but remained at an arbitrary small amount, dropping abruptly to zero at  $x = 0$  and

$x = L$ . This was done to conform with a similar adjustment to the moment of inertia curve. The need for the corrections arose when, in their absence, spurious values of curvature were obtained at the ends because, in general, the curve of inertia tended to converge on zero more rapidly than the curve of bending moment.

The  $K$  factor used to determine  $\beta$  from  $Q$  is defined as

$$K = \frac{\text{average shearing stress in cross section}}{\text{maximum shearing stress in cross section}}$$

or

$$K = \frac{IB}{A \int_0^B y dA} \quad (82)$$

where  $I$  and  $A$  have already been defined, and  $B$  is the total thickness of steel through which the vertical center line passes. (The center line vertical and flat keels were excluded from the calculation of  $K$ ).

The integral of equation (82) was calculated separately for each component of the structure (i. e., the pressure hull, outer hull, deck, etc.), and all the integrals summed to give a value applicable to the entire section. Only members which make major contributions to the submarine's longitudinal strength were considered in the above calculation and in computing sectional moments of inertia. A discussion as to the validity of this practice appears in Appendix D.

slightly so that the ordinates did not approach zero rapidly, but remained at an arbitrary small amount, dropping sharply to zero at  $x = 0$  and  $x = 1$ . This was done in order to give a similar adjustment to the mean-  
 out of inertia curve. The mean for the correction curve when it is  
 applied, various values of ordinates were obtained at the ends of the  
 in general, the curve of inertia tended to converge on zero more rapidly  
 than the curve of bending moment.

The  $K$  factor used to determine  $\theta$  from  $\phi$  is defined as

$$K = \frac{\text{average shearing stress in cross section}}{\text{maximum shearing stress in cross section}}$$

$$K = \frac{\int_0^1 y^2 dy}{\frac{1}{2}}$$

(32)

where  $I$  and  $A$  have already been defined, and  $B$  is the total thickness  
 of steel through which the vertical center line passes. (The center line  
 vertical and the keels were excluded from the calculation of  $K$ ).

The integral of equation (32) was calculated separately for each  
 component of the structure (i.e., the pressure hull, outer hull, deck,  
 etc.), and all the integrals summed to give a value applicable to the  
 entire section. Only members which make major contributions to the  
 structure's longitudinal strength were considered in the above calcu-  
 lation and in computing sectional moments of inertia. A discussion as  
 to the validity of this practice appears in Appendix II.



TABLE VIII

STRUCTURAL CHARACTERISTICS OF 3/8 SCALE

MODEL SUBMARINE

$x/L$	$w$ , lb/ft	$A$ , in <sup>2</sup>	$K$	$I_{xx}$ , in <sup>4</sup>
0.000	400	10	0.701	15,000
.025	1270	15.4	.712	15,000
.050	2550	28.6	.836	15,000
.075	3800	80	.868	24,000
.100	4550	104.8	.535	40,000
.125	5000	116	.487	61,000
.150	5290	122.8	.514	85,000
.175	5680	142	.491	112,000
.200	6150	162.8	.508	135,000
.225	6750	176.6	.554	157,000
.250	7450	183.6	.572	175,000
.275	8000	184.4	.588	192,000
.300	8450	186.6	.590	200,000
.325	8680	192	.592	203,000
.350	8800	197.2	.587	205,000
.375	8910	202.2	.580	216,000
.400	10600	207.6	.586	260,000
.425	11800	268	.526	375,000
.450	11850	313.4	.435	440,000
.475	11400	316.4	.429	444,000
.500	10400	315	.445	442,000
.525	9300	237	.528	365,000
.550	8880	191.8	.612	260,000
.575	8730	190.4	.601	217,000
.600	8700	190	.596	207,000
.625	8700	187.4	.592	201,000
.650	8650	183.8	.591	190,000
.675	8450	180.4	.588	174,000
.700	8000	177.6	.581	160,000
.725	7300	172.4	.572	145,000
.750	6400	165.6	.578	130,000
.775	5450	156.6	.510	114,000
.800	4660	145.4	.498	100,000
.825	3900	130.4	.479	82,000
.850	3060	110.8	.632	60,000
.875	2300	83.4	.711	33,000
.900	1700	56.6	.490	17,000
.925	1330	42.8	.489	12,000
.950	1200	34.0	.547	10,000
.975	1000	28.2	.697	10,000
1.000	400	25	.640	10,000





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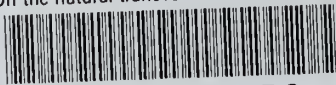
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